

Diamond Oaks  
Transportation Impact Analysis  
Report – City of Visalia

**FINAL REPORT**

Prepared for:

**Diamond Oaks, LP**

Prepared by:



**omni · means**  
ENGINEERS · PLANNERS

**DIAMOND OAKS  
TRANSPORTATION IMPACT ANALYSIS REPORT**

**FINAL REPORT**

**PREPARED FOR:  
DIAMOND OAKS, LP**

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## INTRODUCTION

This report has been prepared to present the results of a Transportation Impact Analysis Report (TIAR) prepared by OMNI-MEANS for a proposed residential development in the City of Visalia. The development, herein called project, is located south of Caldwell Avenue between Burke Street and Ben Maddox Way in the City of Visalia (reference Figure 1).

According to the site plan contained in the Appendix, this proposed project includes 55.7 acres of single- and multi-family dwelling units. Ultimately, this project proposes to add 360 dwelling units, including 203 multi-family units and 157 single-family units. This study will include a multi-phased approach. Phase I includes a total of 99 dwelling units (42 multi-family units, 24 triplex units, and 33 single family units). The additional phases are analyzed under the 2035 cumulative analysis condition.

## EXISTING ROADWAY SYSTEM

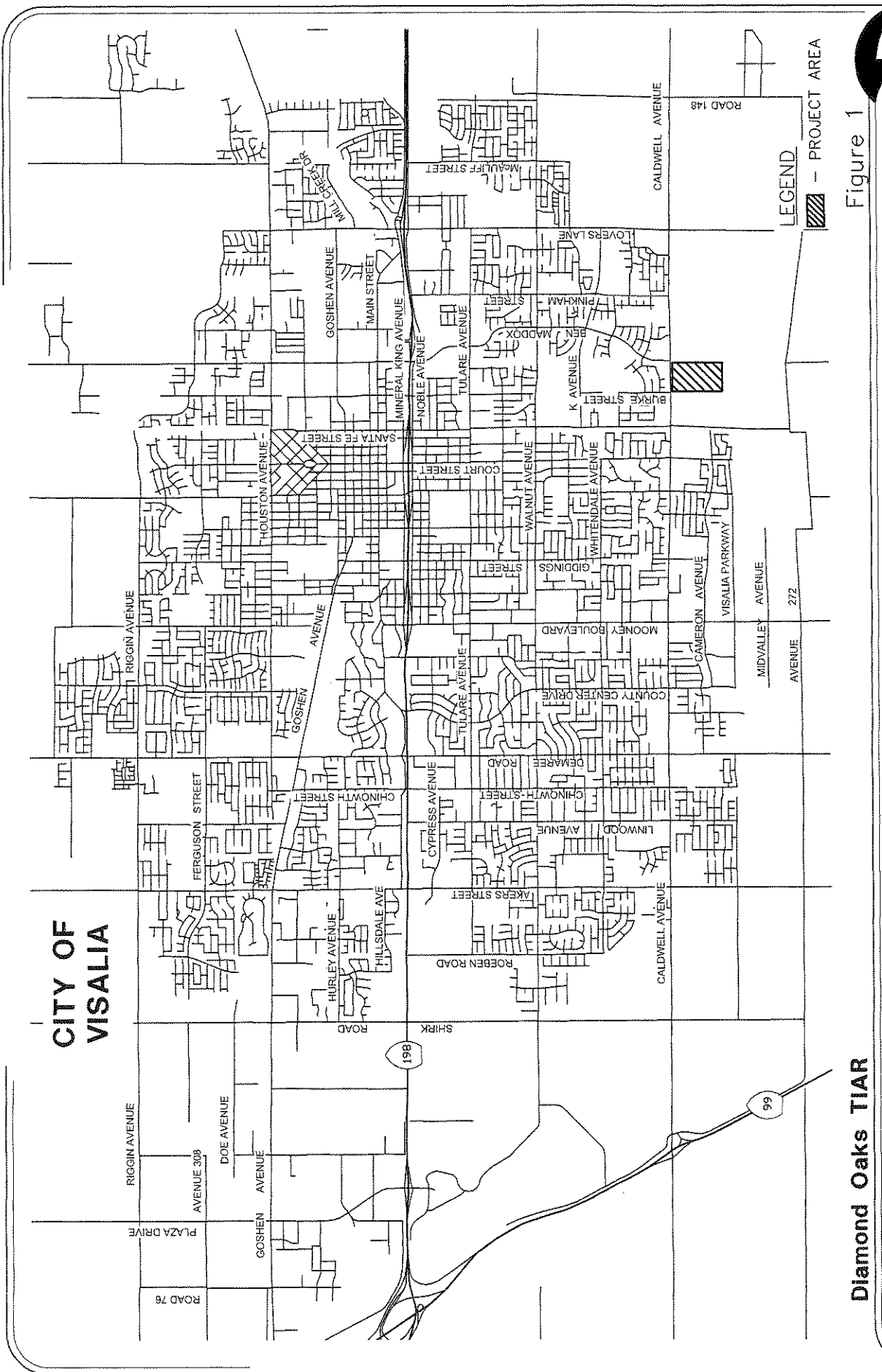
Roadways that provide primary circulation in the vicinity of the project site include Caldwell Avenue, Burke Street and Ben Maddox Way.

*Caldwell Avenue* is a major east-west arterial in Visalia that extends from the Kings County line as Avenue 280, continues through Visalia and terminates in the City of Exeter as Visalia Road. Caldwell Avenue has a varying cross section through the Visalia City Limits, the majority of which is a four-lane divided arterial. West of Burke Street (between Burke Street and Santa Fe Street), Caldwell Avenue is only partially built-out, and varies from a two to four-lane roadway. Along the proposed project frontage (between Burke Street and Ben Maddox Way), Caldwell Avenue currently operates as a two-lane roadway.

*Burke Street* is a non-continuous north-south two-lane collector that extends from Caldwell Avenue north to Monte Vista Avenue within the vicinity of the project. Burke Street then extends from Walnut Avenue, across State Route 198 through downtown Visalia northward to Saint John's Parkway. Burke Street serves residential, commercial and retail uses throughout the corridor. The proposed project will add a south leg to the existing "T" intersection at Caldwell Avenue and Burke Street, which will serve as a project access point.

*Ben Maddox Way* is a north-south four-lane divided arterial roadway that provides access to several residences and business. This roadway extends from Caldwell Avenue north throughout the City of Visalia limits and into the county after providing access at the State Route 198/Ben Maddox Way interchange in central Visalia.

Additional roadways that will provide access in the future include the east-west streets of Russel Avenue, Cameron Avenue, Rialto Avenue and Reese Avenue. These roadways are located south of Caldwell Avenue between Burke Street and Ben Maddox Way, within the proposed subdivision.

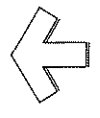


**CITY OF VISALIA**

**Diamond Oaks TIAR**

**Project Vicinity Map**

Figure 1



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## EXISTING TRAFFIC VOLUMES

Based upon OMNI-MEANS' understanding of the project, the following existing intersections were identified as critical intersections for this study.

- Caldwell Avenue/Burke Street
- Caldwell Avenue/Ben Maddox Way

At the study intersections, existing weekday AM and PM peak-hour traffic volume counts were conducted in September 2012 while schools were in session. The AM peak hour is defined as one-hour of peak traffic flow counted between 7:00 AM and 9:00 AM and the PM peak hour is defined as one-hour of peak traffic flow counted between 4:00 PM and 6:00 PM. Figure 2 shows the existing AM and PM peak hour intersection traffic volumes and the existing lane geometrics and control at the study intersections.

## LEVEL OF SERVICE METHODOLOGY

Traffic operations have been quantified through the determination of "Level of Service" (LOS). LOS is a qualitative measure of traffic operating conditions, whereby a letter grade "A" through "F" is assigned to an intersection or roadway segment representing progressively worsening traffic conditions. LOS was calculated for different intersection control types using the methods documented in the *Highway Capacity Manual 2000 (HCM 2000)*. LOS definitions for different types of intersection controls are outlined in Table 1.

The City of Visalia General Plan Circulation Element has designated LOS "D" as the minimum acceptable LOS standard on City facilities in general. In this report, a peak-hour of LOS "D" is taken as the threshold for acceptable traffic operations at all study intersections. All intersection turning movement volumes and LOS worksheets are contained in the Technical Appendix (available upon request).

To determine whether "significance" should be associated with unsignalized intersection LOS, a supplemental traffic signal warrant analysis was also performed. The signal warrant criteria employed for this study are presented in the *Manual on Uniform Traffic Control Devices (MUTCD)*. Specifically, this study utilized the Peak-Hour-Volume Warrant 3 (Urban Areas). Though utilization of this warrant may indicate that signalization would be required, the final decision to provide this improvement should be based on further studies utilizing the additional warrants presented in the MUTCD.

TABLE 1  
LEVEL OF SERVICE CRITERIA  
FOR INTERSECTIONS

LEVEL OF SERVICE	TYPE OF FLOW	DELAY	MANEUVERABILITY	STOPPED DELAY/VEHICLE (SEC)		
				SIGNALIZED	UNSIGNALIZED	ALL-WAY STOP
A	Stable Flow	Very slight delay. Progression is very favorable, with most vehicles arriving during the green phase not stopping at all.	Turning movements are easily made, and nearly all drivers find freedom of operation.	≤ 10.0	≤ 10.0	≤ 10.0
B	Stable Flow	Good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.	Vehicle platoons are formed. Many drivers begin to feel somewhat restricted within groups of vehicles.	>10 and ≤ 20.0	>10 and ≤ 15.0	>10 and ≤ 15.0
C	Stable Flow	Higher delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant, although many still pass through the intersection without stopping.	Back-ups may develop behind turning vehicles. Most drivers feel somewhat restricted	>20 and ≤ 35.0	>15 and ≤ 25.0	>15 and ≤ 25.0
D	Approaching Unstable Flow	The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high volume-to-capacity ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.	Maneuverability is severely limited during short periods due to temporary back-ups.	>35 and ≤ 55.0	>25 and ≤ 35.0	>25 and ≤ 35.0
E	Unstable Flow	Generally considered to be the limit of acceptable delay. Indicative of poor progression, long cycle lengths, and high volume-to-capacity ratios. Individual cycle failures are frequent occurrences.	There are typically long queues of vehicles waiting upstream of the intersection.	>55 and ≤ 80.0	>35 and ≤ 50.0	>35 and ≤ 50.0
F	Forced Flow	Generally considered to be unacceptable to most drivers. Often occurs with over saturation. May also occur at high volume-to-capacity ratios. There are many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors.	Jammed conditions. Back-ups from other locations restrict or prevent movement. Volumes may vary widely, depending principally on the downstream back-up conditions.	> 80.0	> 50.0	> 50.0

References: Highway Capacity Manual 2000

This traffic study generally provides a “planning level” evaluation of traffic operating conditions, which is considered sufficient for California Environmental Quality Act/National Environmental Policy Act (CEQA/NEPA) purposes. This planning level evaluation has, however, incorporated actual heavy-vehicle adjustment factors, peak hour factors, and signal lost-time factors and reports the resulting intersection delays and LOS as estimated using HCM-2000 methodologies. In this study, a general Peak Hour Factor (PHF) of 0.92 has been applied to the analysis of all study intersections under all analysis scenarios.

The HCM-recommended suburban traffic signal default cycle length of 100 seconds has been used for analysis of future signalized intersections, with 4 seconds of “lost time” per critical signal phase. The *Traffix 8.0* integrated computer software program has been utilized to implement the HCM-2000 analysis methodologies.

### EXISTING TRAFFIC OPERATIONS

“Existing” peak-hour intersection traffic operations were quantified applying existing traffic volumes and existing intersection lane geometrics and control (shown on Figure 2). Table 2 presents the “Existing” peak hour intersection LOS.

**TABLE 2  
EXISTING CONDITIONS:  
INTERSECTION LEVELS-OF-SERVICE**

No	Intersection	Control Type	AM Peak Hour			PM Peak Hour		
			Delay (sec/veh)	LOS	Warrant Met?	Delay (sec/veh)	LOS	Warrant Met?
1	Caldwell Avenue/Burke Street	OWSC	18.1	C	No	35.5	E	No
2	Caldwell Avenue/Ben Maddox Way	Signal	20.2	C	--	35.7	D	--

*Legend: OWSC = One-Way-Stop Control. AWSC = All-Way Stop Control.  
Average Delay = Average Intersection Delay for Signalized Intersections.  
Average Delay = Worst-Case Minor Street Approach Delay for OWSC Intersections.  
LOS = Average Intersection Level-of-Service for Signalized Intersections.  
LOS = Worst-Case Minor Street Approach Level-of-Service for OWSC Intersections.  
Warrant = MUTCD Peak-Hour Warrant-3.*

As indicated in Table 2, the intersection at Caldwell Avenue/Burke Street currently operates at LOS “E” conditions during the PM peak hour period under “Existing” conditions. In addition, this unsignalized intersection does not meet the MUTCD Peak Hour Warrant 3 under “Existing” AM or PM peak hour conditions. The final decision to install a traffic signal should be based on further studies utilizing the additional warrants presented in the MUTCD, including, but not limited to:

- Warrant 1; Eight-Hour Vehicular Volume
- Warrant 2; Four-Hour Vehicular Volume
- Warrant 7; Crash Experience

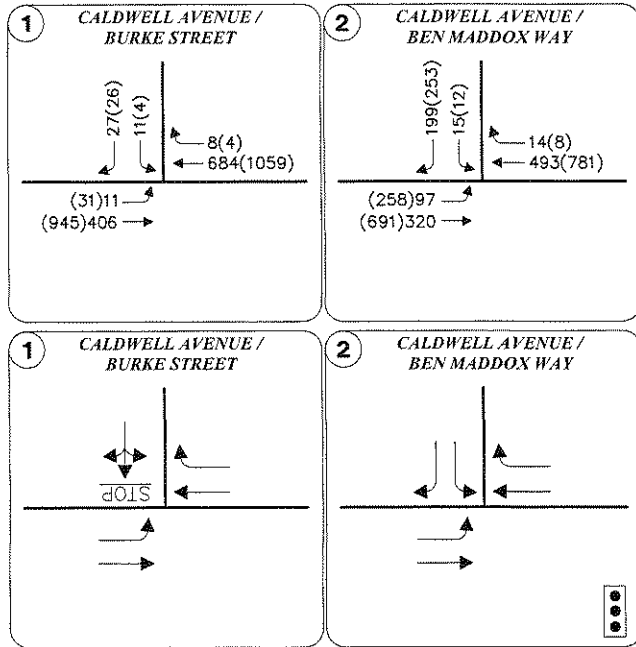
Warrant 1; Eight-hour vehicular volume has a lower threshold of 53 vehicles per hour for each of any eight hours of an average day for the higher volume minor street approach. As indicated by existing traffic counts, there are less than 40 vehicles on the minor street approach for the AM and PM peak hours, indicating that the likelihood of the minor street experiencing volumes that exceed the lower threshold for eight hours is very low. For this reason, it is not likely that the Caldwell Avenue/Burke Street intersection would meet the eight-hour vehicular volume signal warrant.



Warrant 2; Four-hour vehicular volume has a lower threshold of 60 vehicles per hour for each of any four hours of an average day for the higher volume minor street approach. As indicated by existing traffic counts, there are less than 40 vehicles on the minor street approach for the AM and PM peak hours, indicating that the likelihood of the minor street experiencing volumes that exceed the lower threshold for four hours is very low. For this reason, it is not likely that the Caldwell Avenue/Burke Street intersection would meet the four-hour vehicular volume signal warrant.

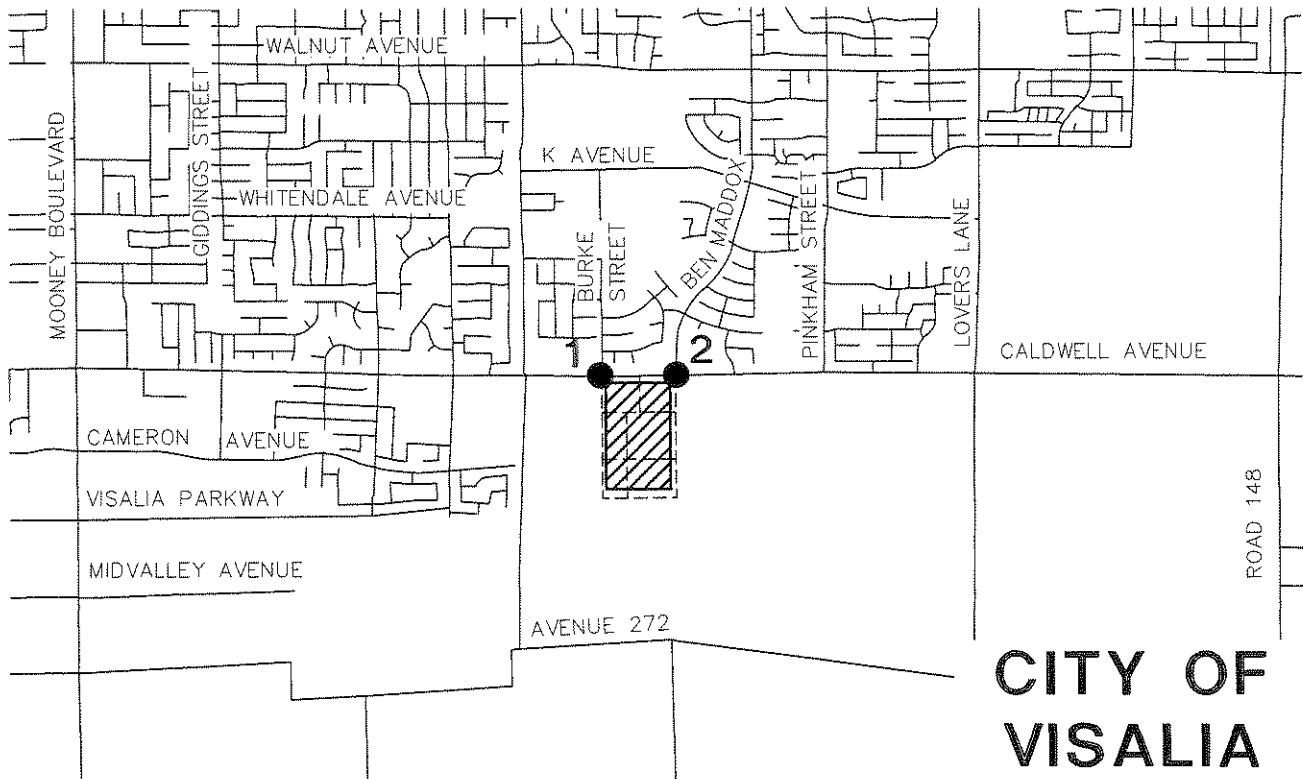
According to the Transportation Injury Mapping System (TIMS), there were no reported collisions at the Caldwell Ave/Burke St intersection during calendar year 2011 (the most recent 12-month period for which data is available from the TIMS website), indicating that the intersection would not meet Warrant 7; Crash Experience. Due to lack of traffic count data, this study does not analyze the four or eight-hour vehicular volume signal warrants.

All mitigation measures are discussed in a subsequent section of this report.



**LEGEND:**

- XX - AM PEAK HOUR TRAFFIC VOLUMES
- (XX) - PM PEAK HOUR TRAFFIC VOLUMES
- ▨ - PROJECT LOCATION

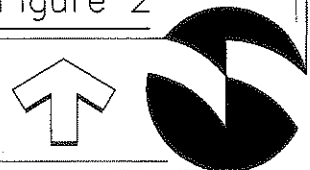


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Figure 2

**Existing Traffic Volumes, Lane Geometrics and Control**



## PHASE I ACCESS

As identified in the introduction, the residential development is located south of Caldwell Avenue between Burke Street and Ben Maddox Way in the City of Visalia. According to the site plan, this proposed project will develop Burke Street south of Caldwell under Phase I as a “Local Through Street Residential Zones” per City of Visalia improvement standard P-17.

## PHASE I TRIP GENERATION

Table 6 identifies the estimated trip generation of the project’s land-uses based upon data presented in *IITE Trip Generation* (8<sup>th</sup> Edition). For this project, trip generation rates for ITE land use codes 210 (single family detached housing), 220 (apartment) and 230 (residential condominium/town house) were applied to obtain the project trips shown in Table 3. As indicated in Table 3, Phase I of this project is estimated to generate 1,136 daily trips, including 94 AM peak hour trips and 121 PM peak hour trips.

**TABLE 3  
PHASE I TRIP GENERATION**

Land Use Category	Unit	Daily Trip Rate/Unit	Weekday AM Peak Hour Rate/Unit			Weekday PM Peak Hour Rate/Unit		
			Total	In %	Out %	Total	In %	Out %
Apartment [ITE Code 220]	Dwelling Unit	7.52	0.54	20%	80%	0.76	65%	35%
Residential Condominium/Town House [ITE Code 230]	Dwelling Unit	5.83	0.71	17%	83%	0.79	67%	33%
Single-Family Detached Housing [ITE Code 210]	Dwelling Unit	11.37	1.00	25%	75%	1.19	63%	37%
Description	Quantity (Units)	Daily Trips	Weekday AM Peak Hour Trips			Weekday PM Peak Hour Trips		
MFDU	84	632	Total	In	Out	Total	In	Out
Triplex Units	24	140	45	9	36	64	42	22
SFDU	32	364	17	3	14	19	13	6
			32	8	24	38	24	14
<b>Total Trips</b>		<b>1,136</b>	<b>94</b>	<b>20</b>	<b>74</b>	<b>121</b>	<b>79</b>	<b>42</b>

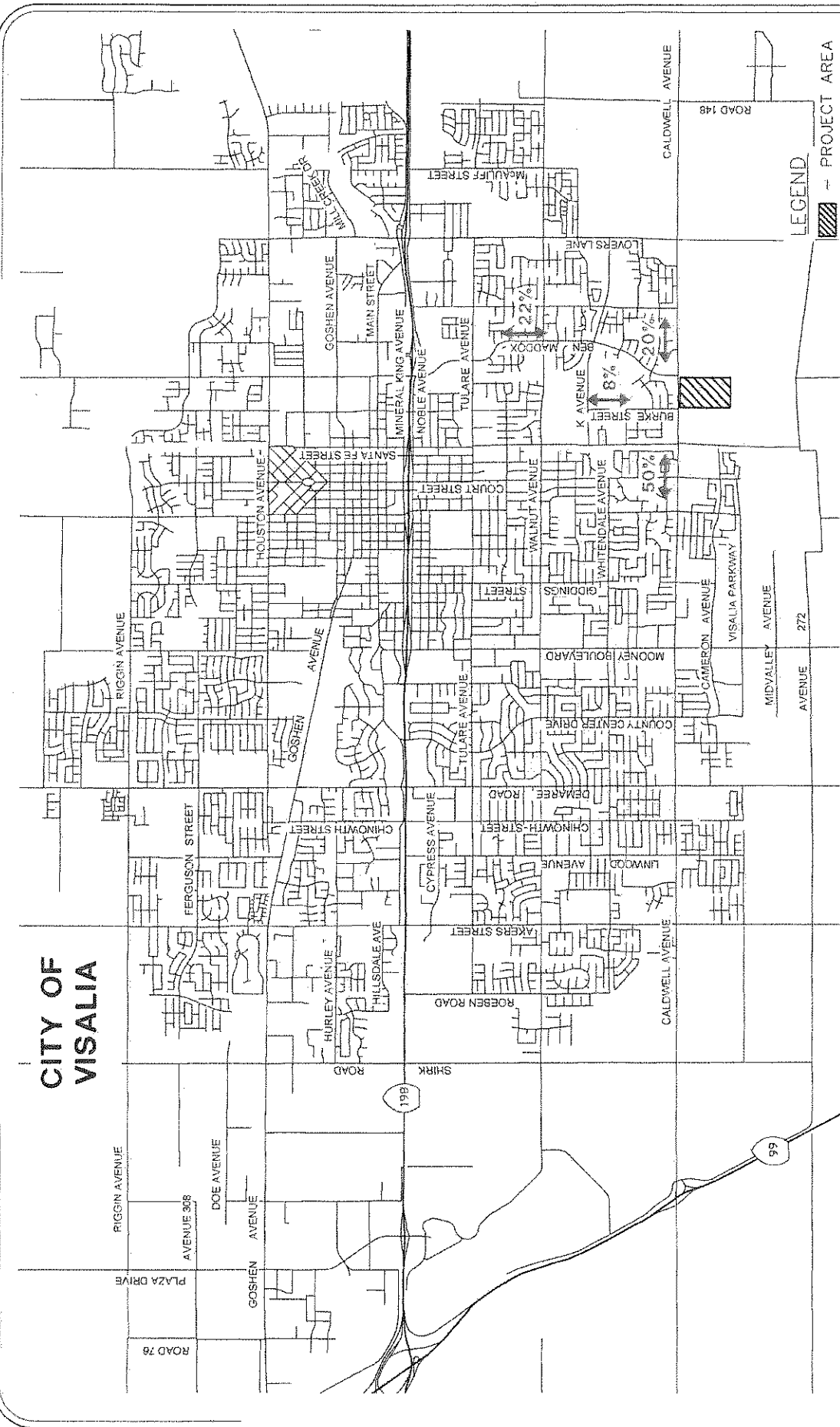
*Note: Errors due to rounding may occur.*

## PHASE I TRIP NATURE, DISTRIBUTION, AND ASSIGNMENT

Phase I of the project is expected to “generate” and “attract” trips throughout the City. Directional trip distribution for project generated trips was estimated based upon a select zone model run from the Tulare County Association of Governments (TCAG), existing traffic flow patterns, geographic location of the project site, and location of other similar destinations. The resulting trip distribution for Phase I project trips is shown on Figure 3 and identified below:

- 50% to/from Caldwell Avenue west of Burke Street;
- 20% to/from Caldwell Avenue east of Ben Maddox Way;
- 8% to/from Burke Street north of Caldwell Avenue; and,
- 22% to/from Ben Maddox Way north of Caldwell Avenue;

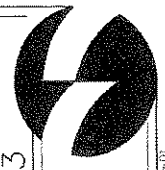
# CITY OF VISALIA



Diamond Oaks TIAR

## Phase I Trip Distribution

Figure 3



## EXISTING PLUS PHASE I CONDITIONS

“Existing plus Phase I” peak-hour intersection operations were quantified utilizing the proposed lane geometrics and control identified in Figure 2. Utilizing *Traffic 8.0* computer software, “Existing plus Phase I” peak hour conditions were simulated by superimposing new trips generated over “Existing” traffic at the study intersections. The resulting “Existing plus Phase I” peak hour intersection traffic volumes and the lane geometrics and control are shown on Figure 4. Table 4 presents the resulting peak hour intersection LOS.

**TABLE 4  
EXISTING PLUS PHASE I CONDITIONS:  
INTERSECTION LEVELS-OF-SERVICE**

No	Intersection	Control Type	AM Peak Hour			PM Peak Hour		
			Delay (sec/veh)	LOS	Warrant Met?	Delay (sec/veh)	LOS	Warrant Met?
1	Caldwell Avenue/Burke Street	TWSC	37.0	E	No	OVRFL	F	No
2	Caldwell Avenue/Ben Maddox Way	Signal	20.9	C	–	40.2	D	–
3	Caldwell Avenue/Edison Street	OWSC	10.9	B	No	18.3	C	No
4	Russel Avenue/Burke Street	TWSC	9.3	A	No	9.2	A	No
5	Cameron Avenue/Burke Street	TWSC	8.5	A	No	8.6	A	No

*Legend:* OWSC = One-Way-Stop Control. TWSC = Two-Ways Stop Control. AWSC = All-Way Stop Control.  
Average Delay = Average Intersection Delay for Signalized Intersections.  
Average Delay = Worst-Case Minor Street Approach Delay for OWSC Intersections.  
LOS = Average Intersection Level-of-Service for Signalized Intersections.  
LOS = Worst-Case Minor Street Approach Level-of-Service for OWSC Intersections.  
Warrant = MUTCD Peak-Hour Warrant-3.  
OVRFL = Overflow conditions (> 100 seconds delay).

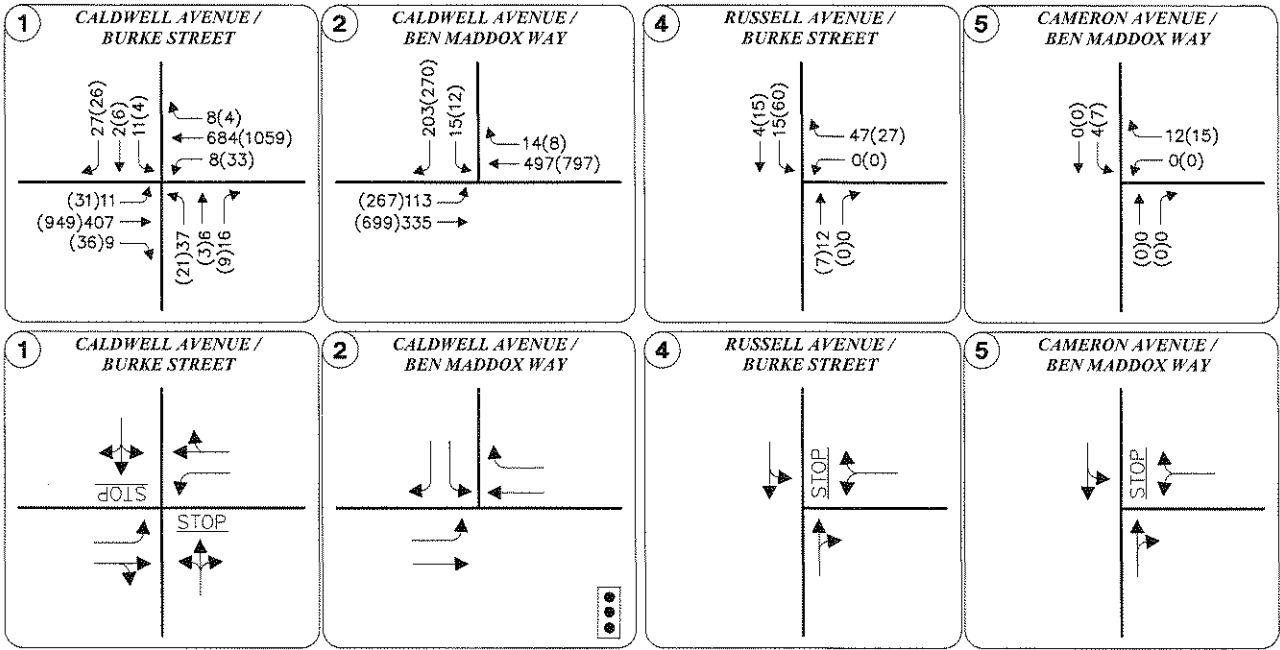
As shown in Table 4, one of the study intersections is projected to operate at a LOS “E/F” conditions during AM and PM peak hour periods under “Existing plus Phase I” conditions scenario. However, the study intersection does not meet the MUTCD Peak Hour Warrant 3 under “Existing plus Phase I” AM and/or PM peak hour conditions. The final decision to install a traffic signal should be based on further studies utilizing the additional warrants presented in the MUTCD, including, but not limited to:

- Warrant 1; Eight-Hour Vehicular Volume
- Warrant 2; Four-Hour Vehicular Volume
- Warrant 7; Crash Experience

The higher volume minor street approach volumes at the Caldwell Avenue/Burke Street intersection are 64 and 36 under Existing plus Phase I AM and PM peak hour conditions, respectively. As previously mentioned the eight-hour and four-hour vehicular volume warrants have lower thresholds of 53 vehicles (for eight hours), and 60 vehicles (for four hours), respectively. Since the AM peak hour higher volume minor street approach is projected to experience volumes just above these lower thresholds, it is not likely that these would hold for four or eight hours of an average day, however, this cannot be 100% verified without collecting traffic data after the first phase of the project is built and occupied.

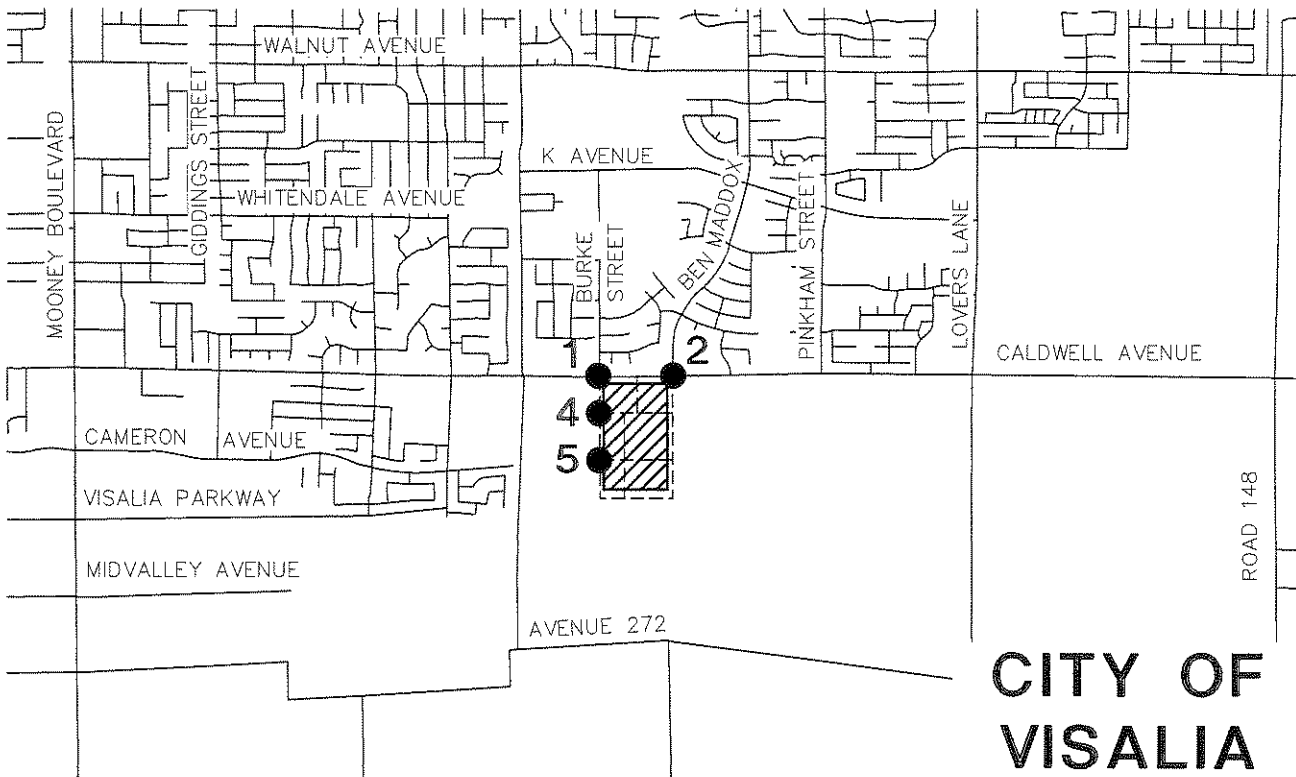
Furthermore, warrant 7; crash experience would need to be evaluated under post project conditions.

All mitigation measures are discussed in a subsequent section of this report.



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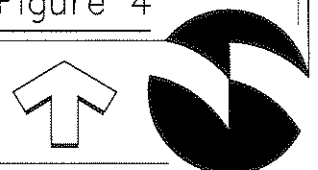
- XX - AM PEAK HOUR TRAFFIC VOLUMES
- (XX) - PM PEAK HOUR TRAFFIC VOLUMES
- ▨ - PROJECT LOCATION



**Diamond Oaks TIAR**

Figure 4

**Existing plus Phase I Traffic Volumes, Lane Geometrics and Control**



## **FUTURE CONDITIONS**

### **GENERAL**

Under Year 2035 conditions, two scenarios were analyzed. Under the first scenario, herein called the “Year 2035 Base” scenario, it is assumed that the City will continue to develop but the project development will not occur. The second scenario, identified as “Year 2035 Base plus Project,” assumes that development of the entire Project will occur.

There are certain improvements within the study area that are assumed to be “in-place” under Year 2035 Base conditions, regardless of whether or not the project is built-out. The following “programmed” improvements, as identified in Phase I of the Measure R expenditure plan, are assumed to be “in-place” under Year 2035 Base conditions.

- Caldwell Avenue (Santa Fe Avenue to Lovers Lane) - Widen to 4-lane divided arterial

OMNI-MEANS worked with TCAG staff to develop future year (2035) traffic volumes utilizing the TCAG Regional Travel Demand Forecast Model, which uses *Cube* software. Although TCAG has a peak hour model, it was not utilized for the future analysis. OMNI-MEANS used the daily directional traffic counts at each leg of the intersection to balance the turning movement counts. The turning movement counts were computed using techniques provided in National Cooperative Highway Research Program (NCHRP 255) through the use of TurnsW32 computer application. Based upon future trip “ins” and “outs” for each leg of the intersection, TurnsW32 runs several iterations to calculate future daily traffic volumes by turning movement. Following this process, OMNI-MEANS checked the forecasted turning movements for reasonableness and made adjustments where necessary. Peak hour traffic volumes were derived from the adjusted daily traffic volumes using typical daily to peak hour traffic relationships.

### **YEAR 2035 BASE CONDITIONS**

“Year 2035 Base” peak-hour intersection traffic operations were quantified applying “Year 2035 Base” traffic volumes and “Year 2035 Base” intersection lane geometrics and control both identified on Figure 5. Table 5 presents the “Year 2035 Base” peak hour intersection LOS.

**TABLE 5  
YEAR 2035 BASE CONDITIONS:  
INTERSECTION LEVELS-OF-SERVICE**

No	Intersection	Control Type	AM Peak Hour			PM Peak Hour		
			Delay (sec/veh)	LOS	Warrant Met?	Delay (sec/veh)	LOS	Warrant Met?
1	Caldwell Avenue/Burke Street	TWSC	33.9	D	No	64.9	F	No
2	Caldwell Avenue/Ben Maddox Way	Signal	23.3	C	—	37.4	D	—

*Legend: TWSC = Two-Way-Stop Control. TWSC = Two-Way Stop Control.  
Average Delay = Average Intersection Delay for Signalized Intersections.  
Average Delay = Worst-Case Minor Street Approach Delay for TWSC Intersections.  
LOS = Average Intersection Level-of-Service for Signalized Intersections.  
LOS = Worst-Case Minor Street Approach Level-of-Service for TWSC/OWSC Intersections.  
Warrant = MUTCD Peak-Hour Warrant-3.  
OVRFL = Overflow conditions (> 100 seconds delay).*

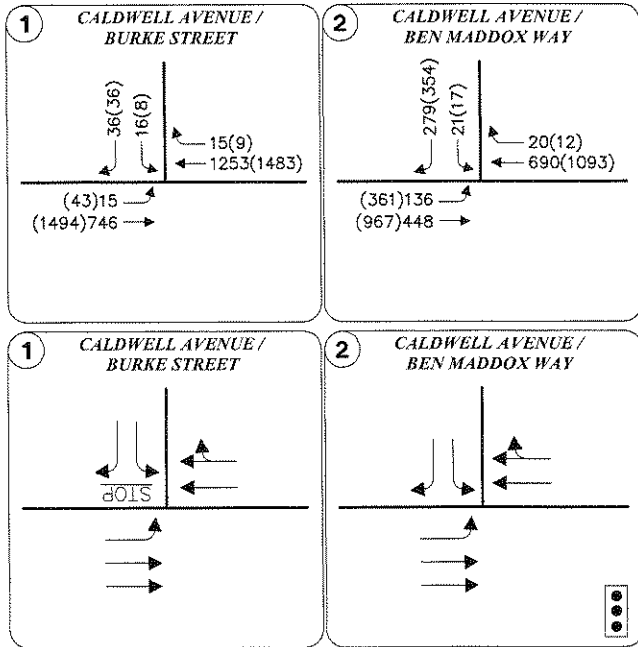
As shown in Table 5, the intersection at Caldwell Avenue/Burke Street is projected to operate at peak hour LOS “F” conditions under PM peak hour periods for “Year 2035 Base” conditions. In addition, this unsignalized intersection at Caldwell Avenue/Burke Street is not forecasted to meet the MUTCD Peak Hour Warrant 3 under “Year 2035 Base” PM peak hour volume conditions. The final decision to install a traffic signal should be based on further studies utilizing the additional warrants presented in the MUTCD, including, but not limited to:

- Warrant 1; Eight-Hour Vehicular Volume
- Warrant 2; Four-Hour Vehicular Volume

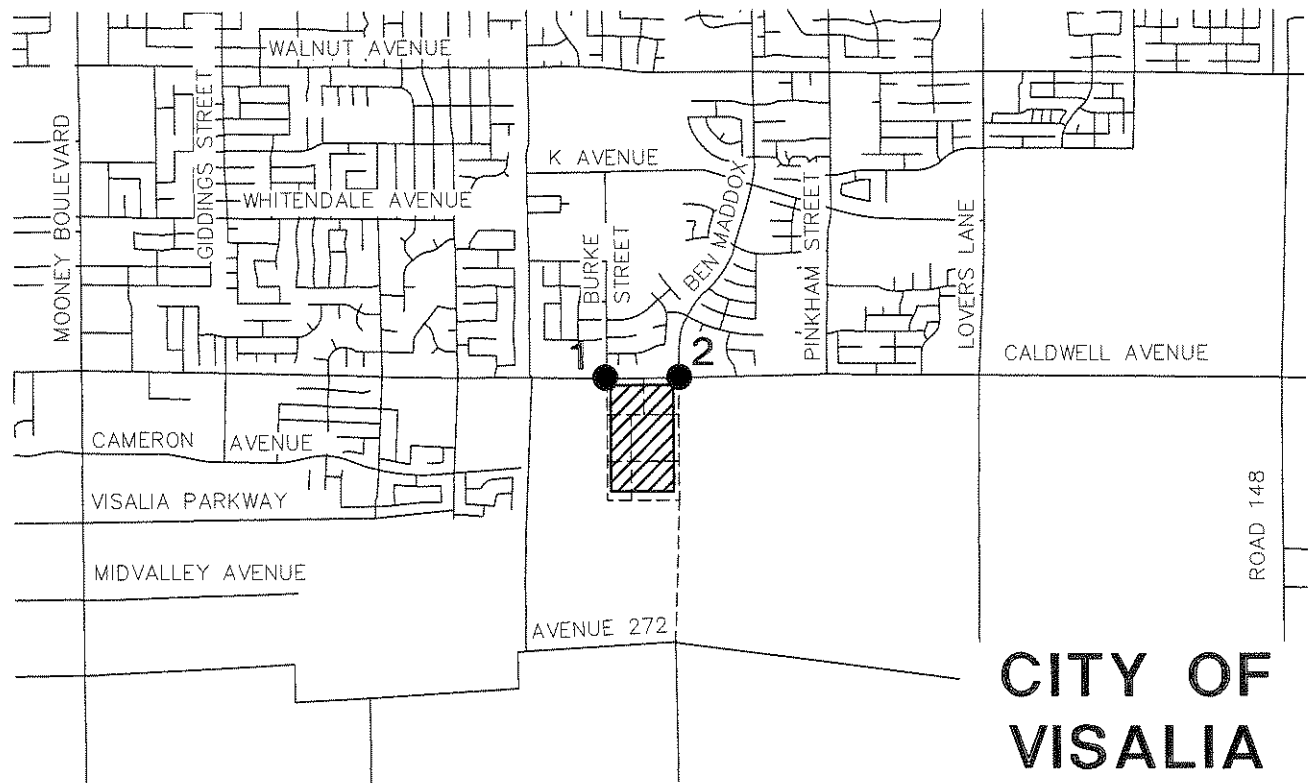
The higher volume minor street approach volumes at the Caldwell Avenue/Burke Street intersection are 52 and 44 under Year 2035 Base AM and PM peak hour conditions, respectively. As previously mentioned the eight-hour and four-hour vehicular volume warrants have lower thresholds of 53 vehicles (for eight hours), and 60 vehicles (for four hours), respectively. Since the AM peak hour higher volume minor street approach is projected to experience volumes just under these lower thresholds, it is not likely that the lower thresholds would be met for four or eight hours of an average day.

All mitigation measures are discussed in a subsequent section of this report.





**LEGEND:**  
 XX - AM PEAK HOUR TRAFFIC VOLUMES  
 (XX) - PM PEAK HOUR TRAFFIC VOLUMES  
 [Hatched Box] - PROJECT LOCATION



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Figure 5

**Year 2035 Base Traffic Volumes, Lane Geometrics and Control**



**YEAR 2035 TRIP GENERATION**

Table 6 identifies the estimated trip generation of the project’s land-uses based upon data presented in *ITE Trip Generation* (8<sup>th</sup> Edition). For this project, trip generation rates for ITE land use codes 220, 230 and 210 were applied to obtain the project trips contained in Table 6. As indicated in Table 6, this project is estimated to generate 2,856 daily trips, including 223 AM peak hour trips and 287 PM peak hour trips for the “Year 2035 Base plus Project” scenario.

**TABLE 6  
PROJECT TRIP GENERATION (PHASES I- IV)**

Land Use Category	Unit	Daily Trip Rate/Unit	Weekday AM Peak Hour Rate/Unit			Weekday PM Peak Hour Rate/Unit		
			Total	In %	Out %	Total	In %	Out %
Apartment [ITE Code 220]	Dwelling Unit	6.80	0.51	20%	80%	0.65	65%	35%
Residential Condominium/ Town House [ITE Code 230]	Dwelling Unit	5.83	0.71	17%	83%	0.79	67%	33%
Single-Family Detached Housing [ITE Code 210]	Dwelling Unit	10.03	0.76	25%	75%	1.01	63%	37%
Description	Quantity (Units)	Daily Trips	Weekday AM Peak Hour Trips			Weekday PM Peak Hour Trips		
			Total	In	Out	Total	In	Out
MFDU	168	1,142	86	17	69	110	72	38
Triplex Units	24	140	17	3	14	19	13	6
SFDU	157	1,574	120	30	90	158	100	58
<b>Total Trips</b>		<b>2,856</b>	<b>223</b>	<b>50</b>	<b>173</b>	<b>287</b>	<b>185</b>	<b>102</b>

*Note: Errors due to rounding may occur.*

**YEAR 2035 BASE PLUS PROJECT CONDITIONS**

“Year 2035 Base plus Project” peak-hour intersection traffic operations were quantified applying “Year 2035 Base plus Project” traffic volumes shown on Figure 6 and “Year 2035 Base plus Project” intersection lane geometrics and control shown on Figure 7. Table 7 presents the “Year 2035 Base plus Project” peak hour intersection LOS.

**TABLE 7  
YEAR 2035 BASE PLUS PROJECT CONDITIONS:  
INTERSECTION LEVELS-OF-SERVICE**

No	Intersection	Control Type	AM Peak Hour			PM Peak Hour		
			Delay (sec/veh)	LOS	Warrant Met?	Delay (sec/veh)	LOS	Warrant Met?
1	Caldwell Avenue/Burke Street	TWSC	OVRFL	F	No	OVRFL	F	No
2	Caldwell Avenue/Ben Maddox Way	Signal	27.2	C	--	45.5	D	--
3	Caldwell Avenue/Edison Street	TWSC	11.3	B	No	15.9	C	No
4	Russel Avenue/Burke Street	TWSC	9.3	A	No	9.2	A	No
5	Cameron Avenue/Burke Street	TWSC	8.5	A	No	8.6	A	No
6	Russel Avenue/Ben Maddox Way	TWSC	9.9	A	No	9.5	A	No
7	Cameron Avenue/Ben Maddox Way	TWSC	8.5	A	No	9.0	A	No
8	Reese Avenue/Bradley Street	TWSC	8.5	A	No	8.5	A	No

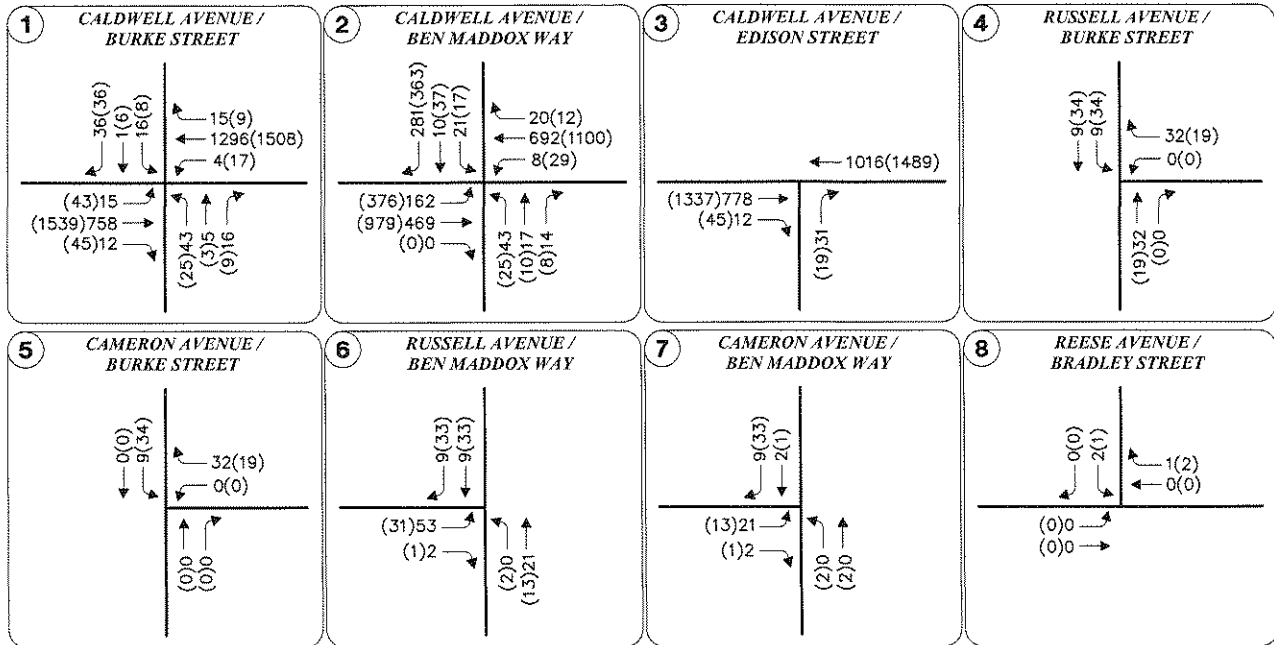
*Legend: TWSC = Two-Way-Stop Control. AWSC = All-Way Stop Control.  
Average Delay = Average Intersection Delay for Signalized Intersections.  
Average Delay = Worst-Case Minor Street Approach Delay for TWSC Intersections.  
LOS = Average Intersection Level-of-Service for Signalized Intersections.  
LOS = Worst-Case Minor Street Approach Level-of-Service for TWSC Intersections.  
Warrant = MUTCD Peak-Hour Warrant-3.  
OVRFL = Overflow conditions (> 100 seconds delay).*

As shown in Table 7, the intersection at Caldwell Avenue/Burke Street is projected to operate at LOS “F” conditions under AM and PM peak hour periods for “Year 2035 Base plus Project” conditions. The unsignalized intersection at Caldwell Avenue/Burke Street is not projected to meet the MUTCD Peak Hour Warrant 3 under projected “Year 2035 Base plus Project” AM and PM peak hour volume conditions. The final decision to install a traffic signal should be based on further studies utilizing the additional warrants presented in the MUTCD, including, but not limited to:

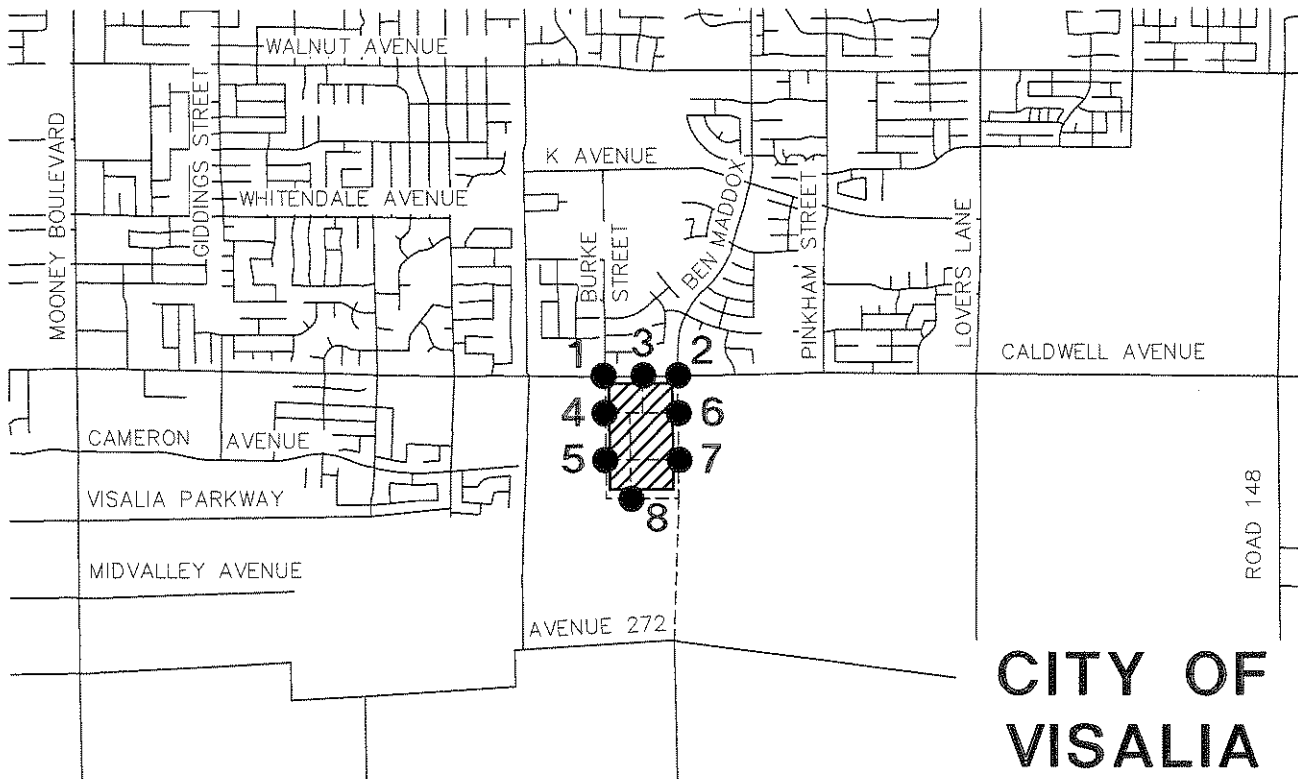
- Warrant 1; Eight-Hour Vehicular Volume
- Warrant 2; Four-Hour Vehicular Volume

The higher volume minor street approach volumes at the Caldwell Avenue/Burke Street intersection are 64 and 53 under Year 2035 Base plus Project AM and PM peak hour conditions, respectively. As previously mentioned the eight-hour and four-hour vehicular volume warrants have lower thresholds of 53 vehicles (for eight hours), and 60 vehicles (for four hours), respectively. Since both the AM and PM peak hour higher volume minor street approach are projected to experience volumes at or near these lower thresholds, further study would be needed under post project cumulative conditions to determine if these warrants are met.

All mitigation measures are discussed in the following section of this report.



**LEGEND:**  
 XX - AM PEAK HOUR TRAFFIC VOLUMES  
 (XX) - PM PEAK HOUR TRAFFIC VOLUMES  
 [Hatched Box] - PROJECT LOCATION



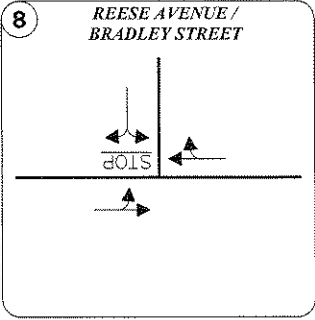
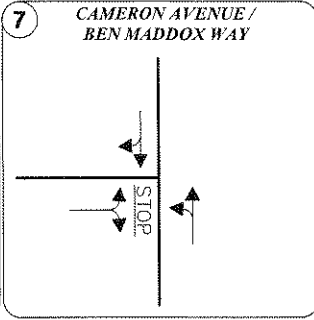
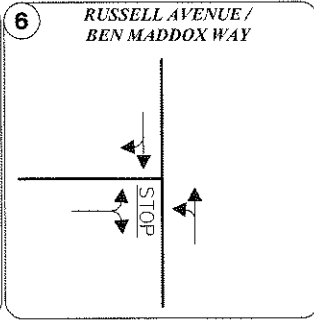
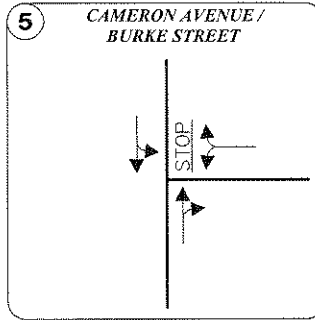
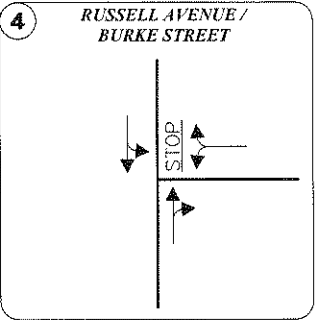
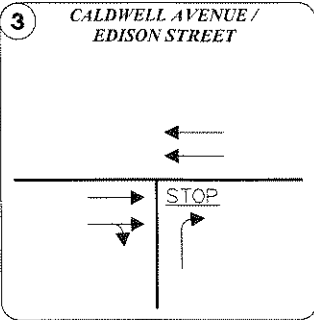
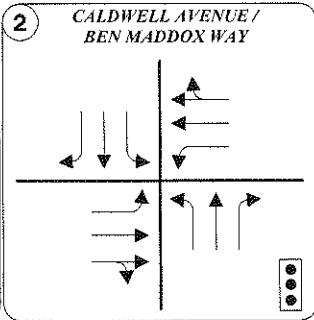
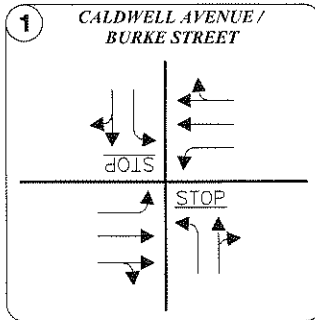
**CITY OF VISALIA**

**Diamond Oaks TIAR**

Figure 6

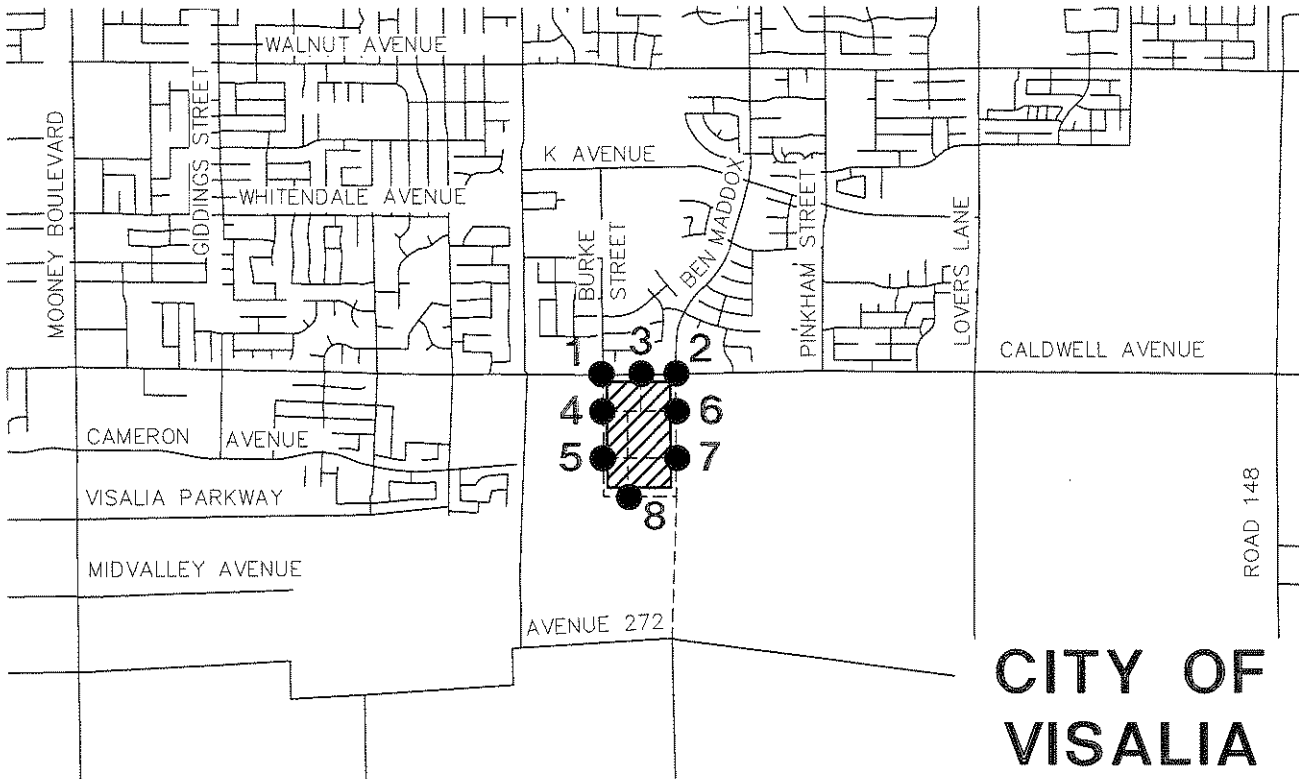
**Year 2035 Base plus Project Traffic Volumes**





LEGEND:

 - PROJECT LOCATION



**CITY OF VISALIA**

Diamond Oaks TIAR

Figure 7

**Year 2035 Base plus Project Lane Geometrics and Control**



## RECOMMENDED MITIGATION MEASURES

This section presents a list of recommended mitigation measures at the study intersections and roadways based upon the results of the analysis presented in this report. All of the study intersections are projected to operate at acceptable LOS “D” conditions or better through 2035 with implementation of the recommended mitigation measures identified below.

At the end of this section, Figure 8 identifies mitigated lane geometrics and control to achieve acceptable operating conditions at the study intersections. Because the mitigation measures are recommended for buildout in Year 2035 and generally do not provide an implementation year, the study intersections requiring mitigation to achieve acceptable LOS should be monitored on a regular basis by the City of Visalia. TCAG has an annual traffic monitoring program that could be used to collect LOS at these intersections.

## EXISTING CONDITIONS

Under “Existing” conditions the following mitigation measures are recommended:

*Caldwell Avenue/Burke Street:* The southbound approach at this intersection is currently operating at PM peak hour LOS “E”. Although not currently warranted based upon CAMUTCD peak hour volume Warrant 3, the installation of a traffic signal at this intersection would result in acceptable LOS during existing peak hour conditions. The traffic signal should be installed to accommodate the ultimate widening of the intersection, or installed concurrent with the ultimate intersection improvements.

The ultimate intersection geometrics would consist of separate left-turns, and shared thru-right lanes on the north/south (Burke Street) legs, and a separate left, thru, and shared thru-right on the east/west (Caldwell Avenue) legs.

If a traffic signal were to be installed prior to the ultimate intersection improvements being constructed, it would operate at acceptable LOS with a shared left-thru-right lane with split or permitted phasing on the north/south approaches, with dedicated left-turn lane, and shared thru-right lanes with protected phasing on the east/west approaches.

## EXISTING PLUS PHASE I CONDITIONS

With the mitigation measures recommended under “Existing” conditions in-place, the study intersections would operate at acceptable peak hour LOS under “Existing plus Phase I” conditions. Under “Existing plus plus Phase I” conditions, the following mitigation measures have been identified:

*Project Driveway intersections:* Based upon the site plan, it is recommended that the future intersections of Russel Avenue/Burke Street and Cameron Avenue/Burke Street operate as stop controlled intersection on the westbound approach with shared turning movements. It is also recommended that the north-south approaches on Burke Street have thru-shared lanes into these access points.

## *Project Access*

It should be noted that the first and second phases of the project, consisting of 175 total units (including a combination of multi-family, single family, and triplex units), will be served by two access points to Caldwell Avenue. Caldwell Avenue will serve as the only roadway which from the site can be accessed. The Caldwell Avenue/Burke Street intersection would provide full access, while right-turn only access

would be permitted at the Caldwell Avenue intersection at Edison Street. Edison Street will connect to Caldwell Avenue approximately mid-way between Ben Maddox Way and Burke Street.

Since the Caldwell Avenue/Burke Street intersection will serve as the only full access point to/from the development site, traffic congestion may result, without the provision for any alternative routes (specifically for left-turn movements in and out of the project site). It is recommended that the installation of a traffic signal to mitigate project related traffic congestion at the Caldwell Avenue/Burke Street intersection be strongly considered. If a traffic signal is not installed, traffic at this location should be monitored on an ongoing basis, as phases of the project are occupied to determine when the intersection operations warrant the installation of a traffic signal.

#### **YEAR 2035 BASE CONDITIONS**

Under “Year 2035 Base” conditions, it is assumed that mitigation measures recommended under “Existing” and “Existing plus Phase I” conditions, in addition to the improvements assumed to be “in-place” by Year 2035, have been implemented. Therefore, no further mitigation measures are necessary.

#### **YEAR 2035 BASE PLUS PROJECT CONDITIONS**

Under “Year 2035 Base plus Project” conditions, it is assumed that mitigation measures recommended under “Existing” and “Existing plus Phase I” conditions, in addition to the improvements assumed to be “in-place” by Year 2035, have been implemented. Under “Year 2035 Base plus Project” the following mitigation measures are recommended:

*Caldwell Avenue/Ben Maddox Way intersection:* This intersection does not require mitigation, however, there are project trips assigned to the ultimate southerly extension of Ben Maddox Way under Year 2035 Base plus Project conditions. Should the project build-out occur prior to the southerly extension of Ben Maddox Way, Burke Street would remain the primary access point to the project. In this case, 104 AM peak hour and 148 PM peak hour trips would be re-assigned to the Caldwell Avenue/Burke Street intersection, resulting in AM and PM peak hour LOS 'B'. The resulting LOS at the Caldwell Avenue/Ben Maddox Way intersection would be LOS 'C', and LOS 'D' during the AM and PM peak hours, respectively.

It is expected that the southerly extension of Ben Maddox Way would be constructed when the adjacent property to the east of the proposed Diamond Oaks subdivision develops. For this reason, assigning a timeframe to the southerly extension of Ben Maddox Way cannot be done with any certainty.

*Caldwell Avenue/Edison Street intersection:* Based upon the site plan, it is recommended that the Caldwell Avenue/Edison Street intersection be a right turn only intersection between Burke Street and Ben Maddox Way. Northbound and westbound left-turn movements should be prohibited at this intersection to maintain the integrity of this major arterial in south Visalia.

*Russell Avenue/Ben Maddox Way and Cameron Avenue/Ben Maddox Way intersections:* Based upon the site plan, it is recommended that these two intersections be stop controlled with shared turning movements on the eastbound approach. In addition, Ben Maddox Way should have a shared left-thru lane on the northbound approaches and a shared thru-right lane on the southbound approaches.

*Reese Avenue/Bradley Street intersection:* Install a stop controlled intersection on the southbound approach of Bradley Street. Furthermore, it is recommended that the eastbound and westbound approaches on Reese Avenue have shared thru-left/right movements into the project site.

**PRO RATA SHARE CALCULATIONS**

Table 8 identifies the pro rata share calculations as documented in the Caltrans *Guide for the Preparation of Traffic Impact Studies* (December 2002). The method for calculating equitable mitigation measures is as follows:

$$P=T/(T_B - T_E)$$

Where:

- P = The equitable share for the proposed project’s traffic impact.
- T = The vehicle trips generated by the project during the peak hour of adjacent State highway facility in vehicles per hour (vph).
- T<sub>B</sub> = The forecasted traffic volume on a impacted State highway facility at the time of general plan build-out (e.g., 20 year model or the furthest future model date feasible), vph.
- T<sub>E</sub> = The traffic volume existing on the impacted State highway facility plus other approved projects that will generate traffic that has yet to be constructed/opened, vph.

**TABLE 8  
PRO RATA SHARE CALCULATIONS**

Intersection	Existing	2035 General Plan Buildout	Project Trips	Pro Rata %
Caldwell Avenue/Burke Street	2,069	3,073	175	17.4

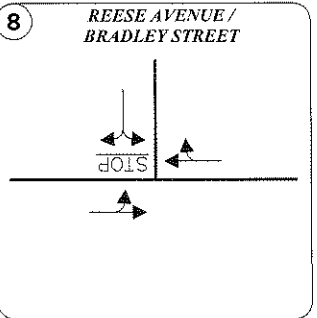
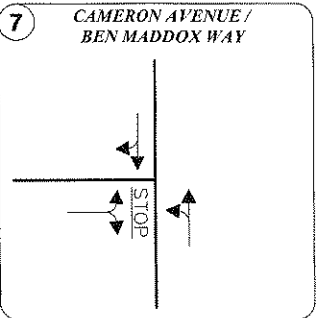
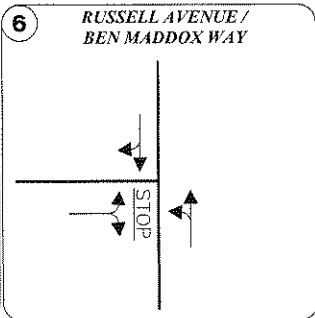
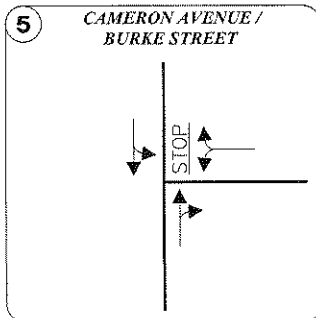
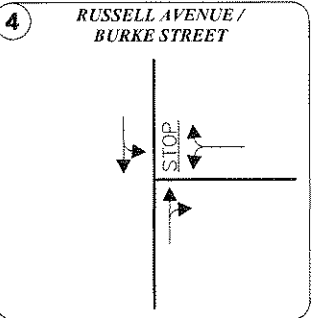
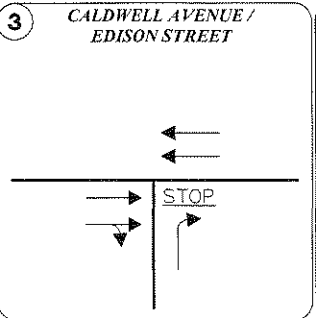
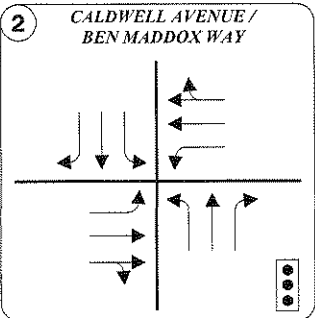
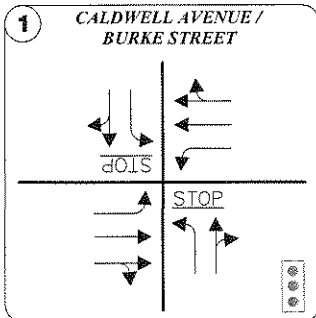
As shown in Table 8, the proposed project will generate a portion of PM peak hour trips that will contribute to the deficiencies identified above.

According to the methodology described in the Caltrans *Guide for the Preparation of Traffic Impact Studies* (December 2002), Table 10 is neither intended as, nor does it establish a legal standard for determining equitable responsibility and cost of the project’s traffic impact; the intent is to provide:

1. A starting point for early discussions to address traffic mitigation equitably;
2. A means for calculating the equitable share for mitigating traffic impacts; and
3. A means for establishing rough proportionality [Dolan vs. City of Tigard, 1994, 512 U.S. 374 (114 S. Ct. 2309)].

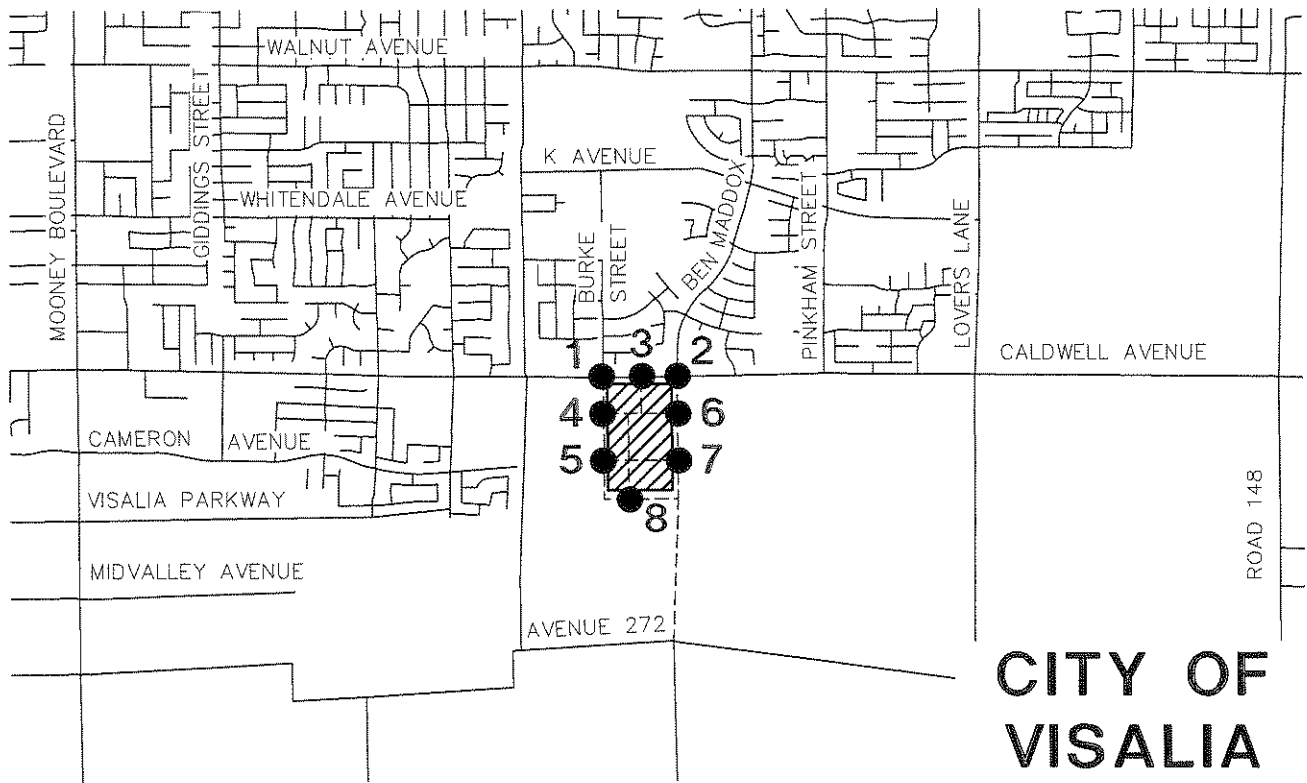
According to the Caltrans’ *Guide for Preparation of Traffic Impact Studies* (December 2002), the method for calculating equitable mitigation measures “. . . is not intended for circumstances where a project proponent will be receiving a substantial benefit from the identified mitigation measure. In these cases, the project should take full responsibility toward providing the necessary infrastructure.”





LEGEND:

- PROJECT LOCATION
- MITIGATION

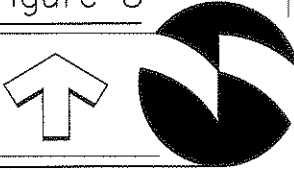


**CITY OF VISALIA**

Diamond Oaks TIAR

Figure 8

**Year 2035 Base plus Project Mitigated Lane Geometrics and Control**



**Appendix**  
*(Site Plan)*

NO.	DATE	BY



**LANE ENGINEERS INC.**  
 CIVIL • STRUCTURAL • SURVEYING  
 979 N. Blackstone Street  
 Tulare, California 93274  
 (559) 688-0283

**DIAMOND OAKS, LP**  
 CALIFORNIA  
 A CONCEPTUAL PHASING PLAN FOR:

PH11  
 SHEET NO. 1  
 DATE: 1/21/13  
 SCALE: 1" = 100'



**LEGAL DESCRIPTION**

Approx. 1/2 section of the southeast quarter of Section 4, Township 26 North, Range 24 East, Merced County, California. Containing 2.00 acres, more or less.

**OWNER/DEVELOPER**

DIAMOND OAKS, LP  
 979 N. Blackstone Street  
 Tulare, CA 93274

**SITE DATA**

APPLICABLE ZONING: R-100  
 PROJECT AREA: 2.00 ACRES  
 PROJECT TYPE: SINGLE-FAMILY RESIDENTIAL  
 PROJECT NAME: DIAMOND OAKS  
 PROJECT LOCATION: 979 N. Blackstone Street, Tulare, CA 93274  
 PROJECT OWNER: DIAMOND OAKS, LP  
 PROJECT ARCHITECT: LANE ENGINEERS INC.  
 PROJECT ENGINEER: [Signature]

**PHASING PLAN SUMMARY**

PHASE	DESCRIPTION	NO. OF LOTS	NO. OF UNITS
PHASE 1	PHASE 1 FAMILY RESIDENTIAL	2	2
PHASE 2	PHASE 2 FAMILY RESIDENTIAL	6	6
PHASE 3	PHASE 3 FAMILY RESIDENTIAL	34	34
PHASE 4	PHASE 4 FAMILY RESIDENTIAL	64	64
TOTAL		106	106

**DIAMOND OAKS  
 CONCEPTUAL PHASING PLAN**



SCALE: 1" = 100'

NO.	DATE	REVISIONS

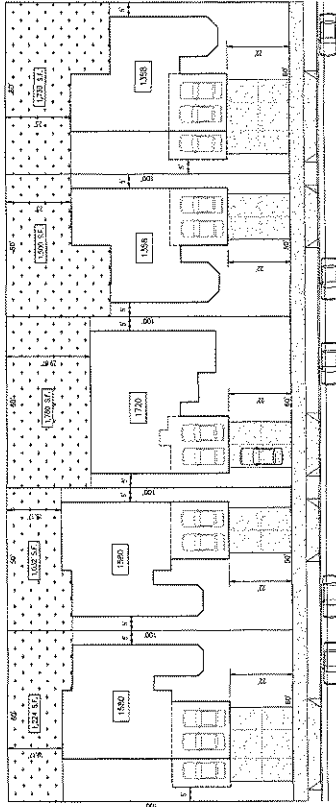


**LANE ENGINEERS INC.**  
 CIVIL, STRUCTURAL • SURVEYING  
 979 N. Blackstone Street  
 Tulare, California 93274  
 (559) 888-9263

**DIAMOND OAKS, LP**  
 A CONCEPTUAL PRO PLAN FOR  
 VISALIA, CALIFORNIA

NAME	AFD
DATE	5-10-12
SCALE	NOTED
CHECKED	LAS
DRAWN	LAS
SHEET	

**PRD11**  
 OF 1 SHEETS  
 05/10/12



**LEGAL DESCRIPTION**

UNDIVIDED PART OF THE 1/4 SECTION 16, T4S, R12E, S12E, COUNTY OF TULARE, CALIFORNIA, BEING THE TRACT OR TRACTS DESCRIBED AS FOLLOWS: ...

**OWNER/DEVELOPER**

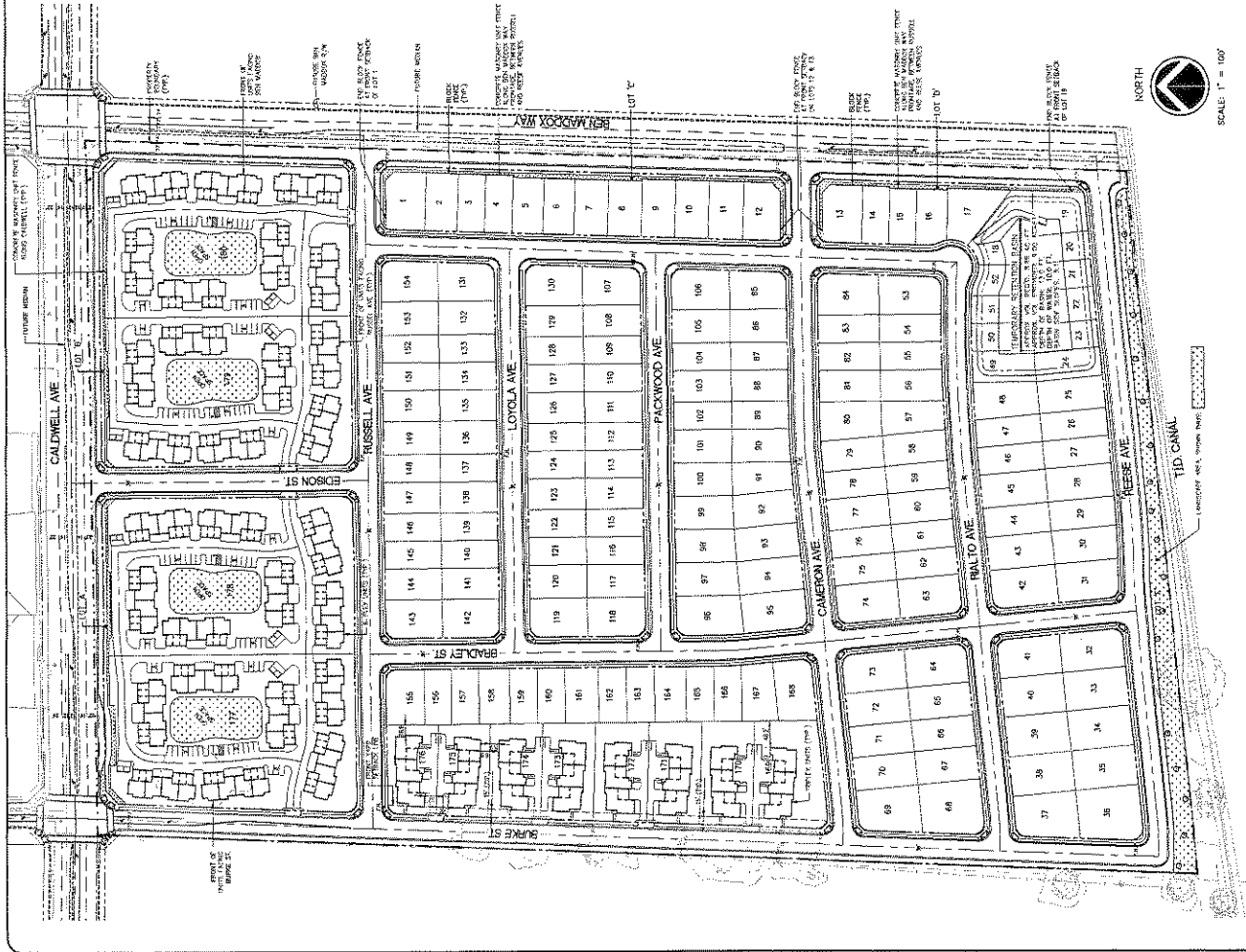
**DIAMOND OAKS, LP**  
 2000 S. VISALIA AVENUE, SUITE 100  
 VISALIA, CALIFORNIA 93277  
 (559) 888-9263

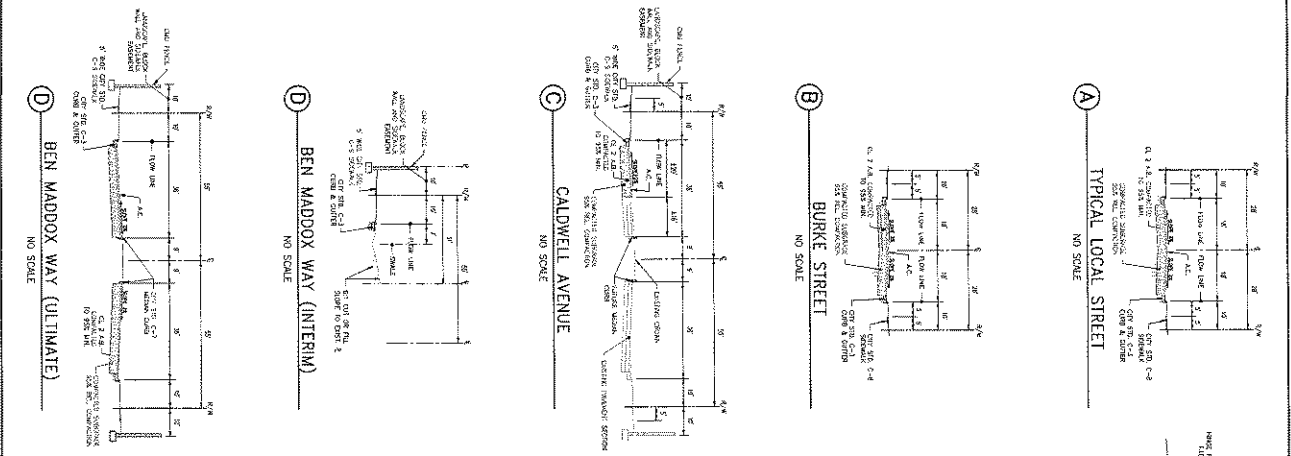
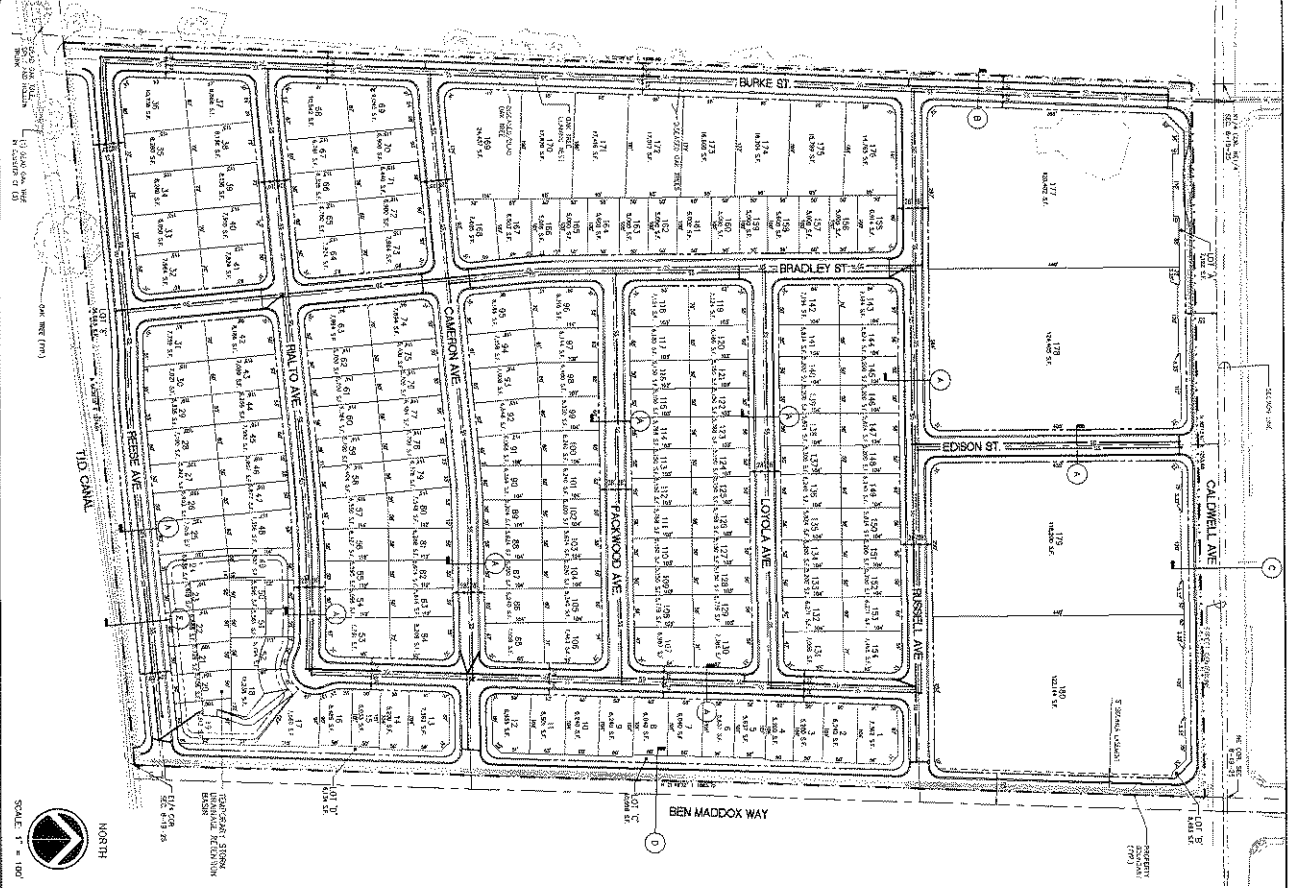
**SITE DATA**

APPLICABLE ZONING: R-1  
 APPLICABLE FORM: R-1-A, R-1-B, R-1-C  
 APPLICABLE FORM: R-1-A, R-1-B, R-1-C  
 APPLICABLE FORM: R-1-A, R-1-B, R-1-C  
 APPLICABLE FORM: R-1-A, R-1-B, R-1-C

**SITE SUMMARY**

**CROSS SITE AREA:** 85.9 ACRES  
**TOTAL NUMBER OF LOTS (1-198):** 198  
**TOTAL GROSS LOT AREA:** 24,022 ACRES  
**TOTAL NUMBER OF SINGLE LOTS (100-198):** 99  
**TOTAL TRUCKER LOT AREA:** 14,001 S.F.  
**TOTAL TRUCKER LOT AREA:** 3.22 ACRES  
**TOTAL NUMBER OF TRUCK LOTS (177-198):** 22  
**TOTAL TRUCK LOT AREA:** 46,143 S.F.  
**TOTAL TRUCK LOT AREA:** 1.04 ACRES  
**TOTAL NUMBER OF TRUCK LOTS (1-4):** 4  
**TOTAL TRUCK LOT AREA:** 56,810 S.F.  
**TOTAL TRUCK LOT AREA:** 1.29 ACRES  
**TOTAL TRUCKER LOT AREA:** 500 UNITS  
**TOTAL TRUCKER LOT AREA:** 42.84 ACRES  
**TOTAL TRUCKER LOT AREA:** 29,806 ACRES  
**TOTAL TRUCKER LOT AREA:** 680 UNITS  
**TOTAL TRUCKER LOT AREA:** 2.88 ACRES  
**TOTAL TRUCKER LOT AREA:** 7.48 ACRES  
**TOTAL TRUCKER LOT AREA:** 117 UNITS  
**TOTAL TRUCKER LOT AREA:** 5.59 ACRES  
**TOTAL TRUCKER LOT AREA:** 17.78 ACRES  
**TOTAL TRUCKER LOT AREA:** 199 UNITS  
**TOTAL TRUCKER LOT AREA:** 488 UNITS





## DIAMOND OAKS VESTING TENTATIVE SUBDIVISION MAP FOR A PLANNED RESIDENTIAL DEVELOPMENT

**NOTE:**  
IT IS THE INTENTION OF THE SUBDIVIDER  
TO FILE THE DECLARATION AND SUBDIVISION MAPS  
ON THIS TENTATIVE MAP.

**SITE SUMMARY**

GRASSY SURF AREA: 24,514 SQUARE FEET  
 TOTAL NUMBER OF SPUR LOTS (1-188): 188  
 TOTAL SPUR LOT AREA: 1,008,228 SQUARE FEET  
 TOTAL SPUR LOT PERIMETER: 2,548 FEET  
 TOTAL NUMBER OF TRAPEZOIDAL LOTS (189-193): 5  
 TOTAL TRAPEZOIDAL LOT AREA: 1,600 SQUARE FEET  
 TOTAL TRAPEZOIDAL LOT PERIMETER: 1,122 FEET  
 TOTAL TRAPEZOIDAL LOT PERIMETER: 1,122 FEET  
 TOTAL TRAPEZOIDAL LOT AREA: 1,600 SQUARE FEET  
 TOTAL TRAPEZOIDAL LOT PERIMETER: 1,122 FEET  
 TOTAL TRAPEZOIDAL LOT AREA: 1,600 SQUARE FEET  
 TOTAL TRAPEZOIDAL LOT PERIMETER: 1,122 FEET

**LEGAL DESCRIPTION**  
 BLOCK 2, LOTS 1-188 OF THE VESTING TENTATIVE SUBDIVISION MAP FOR A PLANNED RESIDENTIAL DEVELOPMENT, DIAMOND OAKS, TULARE COUNTY, CALIFORNIA, AS SHOWN ON THE ATTACHED MAPS AND RECORDS.

**OWNER/DEVELOPER**  
 DIAMOND OAKS, LP  
 1800 WEST 23RD STREET  
 TULARE, CALIFORNIA 93274

**SITE DATA**  
 2.0 ACRES  
 188 LOTS  
 1,008,228 S.F.  
 2,548 FEET PERIMETER  
 1,122 FEET PERIMETER (TRAPEZOIDAL LOTS)  
 1,600 S.F. AREA (TRAPEZOIDAL LOTS)  
 1,122 FEET PERIMETER (TRAPEZOIDAL LOTS)  
 1,600 S.F. AREA (TRAPEZOIDAL LOTS)

DATE	APR 10-12
SCALE	AS SHOWN
DESIGNER	LANE ENGINEERS INC.
PROJECT	SP11
NO.	0132

VESTING TENTATIVE SUBDIVISION MAP FOR:  
**DIAMOND OAKS, LP**  
 VISALIA, CALIFORNIA

**LANE ENGINEERS INC.**  
 CIVIL • STRUCTURAL • SURVEYING  
 979 N. Blackstone Street  
 Tulare, California 93274  
 (559) 689-5263



NO.	DATE	REVISIONS

**Technical Appendix**  
*(Available Upon Request)*

-----  
 Diamond Oaks TIR  
 55-2454-01/CN 1639  
 Existing AM Peak Hour + Phase I  
 -----  
 Trip Generation Report

Forecast for AM Peak

Zone #	Subzone	Amount	Units	Rate		Trips		Trips Total	
				In	Out	In	Out	In	Out
1	Zone 1 AM Peak	94.00	AM Peak	0.21	0.79	20	74	94	100.0
Zone 1 Subtotal						20	74	94	100.0
TOTAL						20	74	94	100.0

-----  
 Diamond Oaks TIR  
 55-2454-01/CN 1639  
 Existing AM Peak Hour + Phase I  
 -----  
 Scenario Report

AM Phase I

Default Command  
 AM Peak  
 Phase I  
 Default Impact Fee  
 AM Peak  
 Trip Generation:  
 Existing plus Phase I  
 Paths:  
 Default Path  
 Routes:  
 Default Route  
 Configuration:  
 Default Configuration

Trip Distribution Report

Zone	Percent Of Trips Default								
	1	2	3	4	6	7	8	10	11
1	10.0	8.0	22.0	20.0	5.0	5.0	5.0	20.0	5.0

Turning Movement Report  
 AM Peak

Volume Type	Northbound		Southbound		Eastbound		Westbound		Total
	Left	Thru Right	Left	Thru Right	Left	Thru Right	Left	Thru Right	
<b>#1 Caldwell Avenue/Burke Street</b>									
Base	0	0	11	0	27	11	406	0	684
Added	37	6	16	0	2	0	1	9	8
Total	37	6	27	0	29	407	9	9	692
<b>#2 Caldwell Avenue/Ben Maddox Way</b>									
Base	0	0	15	0	199	97	320	0	493
Added	0	0	0	0	4	16	15	0	4
Total	0	0	15	0	203	335	0	0	497
<b>#3 Caldwell Avenue/Edison Street</b>									
Base	0	0	0	0	0	0	417	0	592
Added	0	0	15	0	0	0	16	1	8
Total	0	0	15	0	0	0	433	1	700
<b>#4 Russell Ave/Burke St</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	12	0	15	4	0	0	0	47
Total	0	12	0	15	4	0	0	0	78
<b>#5 Cameron Ave/Burke St</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	4	0	0	0	0	12
Total	0	0	0	4	0	0	0	0	16
<b>#6 Russell Ave/Ben Maddox Way</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0
<b>#7 Cameron Ave/Ben Maddox Way</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0
<b>#8</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0



Dimond Oaks TIR  
 55-2454-01/CN 1639  
 Existing AM Peak Hour + Phase I

Impact Analysis Report  
 Level of Service

Intersection	Base Del/V/LOS	V/C	Future Del/V/LOS	V/C	Change In
# 1 Caldwell Avenue/Burke Street	18.2	0.071	37.0	0.300	+18.801 D/V
# 2 Caldwell Avenue/Ben Raddox Way	19.8	0.539	20.5	0.556	+ 0.664 D/V
# 3 Caldwell Avenue/Edison Street	0.0	0.000	10.9	0.024	+10.886 D/V
# 4 Russell Ave/Burke St	0.0	0.000	9.3	0.015	+ 9.303 D/V
# 5 Cameron Ave/Burke St	0.0	0.000	8.5	0.004	+ 8.514 D/V

Dimond Oaks TIR  
 55-2454-01/CN 1639  
 Existing AM Peak Hour + Phase I

Signal Warrant Summary Report

Intersection	Base Met [Del / V/c]	Future Met [Del / V/c]
# 1 Caldwell Avenue/Burke Street	No / No	No / No
# 3 Caldwell Avenue/Edison Street	No / No	No / No
# 4 Russell Ave/Burke St	No / No	No / No
# 5 Cameron Ave/Burke St	No / No	No / No

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #1 Caldwell Avenue/Burke Street

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0

Initial Vol: 0 0 0 0 11 0 27 11 406 0 0 684 8

ApproachDel: xxxxxx 18.2 xxxxxx xxxxxx

Approach[southbound][lanes=1][control=stop Sign]

Signal Warrant rule #1: [vehicle-hours=0.2]

FALL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant rule #2: [approach volume=38]

FALL - Approach volume less than 100 for one lane approach.

Signal Warrant rule #3: [approach count=3][total volume=1147]

SUCCESS - Total volume greater than or equal to 650 for intersection

with less than four approaches.

SIGNAL WARRANT DISCARTMER

The peak hour warrant analysis in this report is not intended to replace

a rigorous and complete traffic signal warrant analysis by the responsible

jurisdiction. Consideration of the other signal warrants, which is beyond

the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #1 Caldwell Avenue/Burke Street

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0

Initial Vol: 0 0 0 0 11 0 27 11 406 0 0 684 8

ApproachDel: xxxxxx 18.2 xxxxxx xxxxxx

Approach[southbound][lanes=1][control=stop Sign]

Signal Warrant rule #1: [vehicle-hours=0.2]

FALL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant rule #2: [approach volume=38]

FALL - Approach volume less than 100 for one lane approach.

Signal Warrant rule #3: [approach count=3][total volume=1147]

SUCCESS - Total volume greater than or equal to 650 for intersection

with less than four approaches.

SIGNAL WARRANT DISCARTMER

The peak hour warrant analysis in this report is not intended to replace

a rigorous and complete traffic signal warrant analysis by the responsible

jurisdiction. Consideration of the other signal warrants, which is beyond

the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 1 0 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	37 6 16 11 2 27	11 2 27	11 407 9	8 684 8
Approach Del:	37.0	22.0	xxxxxx	xxxxxx

Approach[northbound][lanes=1][control=stop sign]  
 Signal Warrant Rule #1: [vehicle-hours=0.6]  
 FAIL - Vehicle-hours less than 4 for one lane approach.  
 Signal Warrant Rule #2: [approach volume=59]  
 FAIL - Approach volume less than 100 for one lane approach.  
 Signal Warrant Rule #3: [approach count=4][total volume=1226]  
 SUCCEEDED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Approach[southbound][lanes=1][control=stop sign]  
 Signal Warrant Rule #1: [vehicle-hours=0.2]  
 FAIL - Vehicle-hours less than 4 for one lane approach.  
 Signal Warrant Rule #2: [approach volume=40]  
 FAIL - Approach volume less than 100 for one lane approach.  
 Signal Warrant Rule #3: [approach count=4][total volume=1226]  
 SUCCEEDED - Total volume greater than or equal to 800 for intersection with four or more approaches.

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report [Urban]

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 1 0 0	0 0 1 0 0	1 0 0 1 0	1 0 0 1 0
Initial Vol:	37 6 16 11 2 27	11 2 27	11 407 9	8 684 8
Major Street Volume:		1127		
Minor Approach Volume Threshold:		59		

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Delay Signal Warrant Report  
 Intersection #3 Caldwell Avenue/Edison Street  
 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L T R	L T R	L T R	L T R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 417	0 0 692
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Peak Hour Volume Signal Warrant Report (Urban)  
 Intersection #3 Caldwell Avenue/Edison Street  
 Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L T R	L T R	L T R	L T R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 1 0 0
Initial Vol:	0 0 0 0	0 0 0 0	0 0 417	0 0 692
ApproachDel:	xxxxxx	xxxxxx	xxxxxx	xxxxxx

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
Intersection #3 Caldwell Avenue/Edison Street  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0  
Initial Vol: 0 0 0 15 0 0 0 0 0 0 433 1 0 700 0  
Approachbel: 18.9 xxxxxx xxxxxx

Approach(northbound)[lanes=1][control=stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=0.8]  
Signal Warrant Rule #2: [approach volume=15]  
Signal Warrant Rule #3: [approach count=3][total volume=1149]  
SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report (Urban)  
Intersection #3 Caldwell Avenue/Edison Street  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0  
Initial Vol: 0 0 0 15 0 0 0 0 0 0 433 1 0 700 0  
Major Street Volume: 1134  
Minor Approach Volume: 45  
Minor Approach Volume Threshold: 186

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
\*\*\*\*\*  
Intersection #4 Russell Ave/Burke St  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Uncontrolled Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 1  
Initial Vol: 0  
Approachel: 0.0 0.0 0.0 0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: Inf

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an  
"indicator" of the likelihood of an unsignalized intersection warranting  
a traffic signal in the future. Intersections that exceed this warrant  
are probably more likely to meet one or more of the other volume based  
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace  
a rigorous and complete traffic signal warrant analysis by the responsible  
jurisdiction. Consideration of the other signal warrants, which is beyond  
the scope of this software, may yield different results.

Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
\*\*\*\*\*  
Intersection #4 Russell Ave/Burke St  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Uncontrolled Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 1  
Initial Vol: 0  
Approachel: 0.0 0.0 0.0 0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: Inf

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an  
"indicator" of the likelihood of an unsignalized intersection warranting  
a traffic signal in the future. Intersections that exceed this warrant  
are probably more likely to meet one or more of the other volume based  
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace  
a rigorous and complete traffic signal warrant analysis by the responsible  
jurisdiction. Consideration of the other signal warrants, which is beyond  
the scope of this software, may yield different results.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #4 Russell Ave/Burke St

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 1

Initial Vol: 0 12 0 15 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 47

Approach: 9.3 8.7  
Major Street Volume: 47

Signal Warrant Rule #1: [vehicle-hours=0.0]  
Signal Warrant Rule #2: [approach volume<12]  
Signal Warrant Rule #3: [approach count<3] [total volume<78]  
FAIL - Total volume less than 650 for intersection with less than four approaches.

Approach[northbound][lanes=1][control=stop sign]  
Signal Warrant Rule #1: [vehicle-hours=0.0]  
FAIL - Vehicle-hours less than 4 for one lane approach.  
Signal Warrant Rule #2: [approach volume<19]  
FAIL - Approach volume less than 300 for one lane approach.  
Signal Warrant Rule #3: [approach count<3] [total volume<78]  
FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #5 Cameron Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 1 0	1 0 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: \*inf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report [Other]

Intersection #5 Cameron Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 1 0	1 0 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: \*inf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.



Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #5 Cameron Ave/Burke St

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop sign Stop sign Uncontrolled Uncontrolled

Lanes: 0 0 0 1 0 1 0 0 0 0 0 0 0 0 0 0 1

Initial Vol: 0 0 0 0 4 0 0 0 0 0 0 0 0 0 0 0 12

Approaches: xxxxxx 8.5 xxxxxx xxxxxx

Approach(southbound)[lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0.0]

Signal Warrant Rule #2: [approach volume=4]

Signal Warrant Rule #3: [approach count=2][total volume=16]

FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #5 Cameron Ave/Burke St

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop sign Stop sign Uncontrolled Uncontrolled

Lanes: 0 0 0 1 0 1 0 0 0 0 0 0 0 0 0 0 1

Initial Vol: 0 0 0 0 4 0 0 0 0 0 0 0 0 0 0 0 12

Approaches: xxxxxx 12 xxxxxx xxxxxx

Approach(southbound)[lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0.0]

Signal Warrant Rule #2: [approach volume=4]

Signal Warrant Rule #3: [approach count=2][total volume=16]

FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Unsignalized Method  
Base Volume Alternative

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R L T R L T R  
KeyVeh: 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3# 3#  
Grads: 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0# 0#  
Pedestrian Walk Speed: 4.00 feet/sec 0 0 0 0  
LaneWidth: 12 feet 12 feet 12 feet 12 feet 12 feet 12 feet  
Time Period: 0.25 hour  
Upstream Signals:  
Link Index: #6  
Dist(miles): 0.00  
Speed (mph): 0.00  
SignalIndex: #2  
Cycle Time: 0 secs  
InitVolume: 0 0 0 0  
Saturation: 0 0 0 0  
ArrivalType: 0 0 0 0  
G/C: 0.00 0.00

\*\*\* Computation 1: Time for Queue to Clear at Each Upstream Intersection  
P: 0.000 0.000  
gq1: 0.00 0.00  
gq2: 0.00 0.00

\*\*\* Computation 2: Time Intersection Blocked Because of Upstream Platoons  
alpha: 0.000  
brta: 0.000  
ta (secs): 0.000  
F: 0.000 0.000

\*\*\* Computation 3: Platoon Event Periods  
pden/psdbr: 0.000/0.000/Unconstrained

\*\*\* Computation 4: Conflicting Flows During Each Unblocked Period  
InitCrflVol: 1278 1217 441 1213 1213 748 752 xxxxx xxxxx 0 xxxxx xxxxx  
AdjCrflVol: 1278 1217 441 1213 1213 748 752 xxxxx xxxxx 0 xxxxx xxxxx  
UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
ConflictVol: 1228 1217 441 1213 1213 748 752 xxxxx xxxxx 0 xxxxx xxxxx

\*\*\* Computation 5: Capacity for Subject Movement During Unblocked Period  
InitPotCap: 154 180 614 200 181 411 844 xxxxx xxxxx 1604 xxxxx xxxxx  
UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Potent Cap.: 154 180 614 200 181 411 844 xxxxx xxxxx 1604 xxxxx xxxxx

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Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*  
Average Delay (sec/veh): 0.7 Worst Case Level of Service: C [18.2]  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R L T R L T R L T R  
Control: Stop Sign Step Sign Uncontrolled Uncontrolled Uncontrolled  
Rights: Include Include Include Include Include Include  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0  
Volume Module:  
Base Vol: 6 0 0 11 0 27 11 406 0 0 684 8  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 11 0 27 11 406 0 0 684 8  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 0 0 0 12 0 29 12 441 0 0 743 9  
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0  
FinalVolume: 0 0 0 12 0 29 12 441 0 0 743 9

Critical Gap Module:  
Critical Gap: 7.1 6.2 6.4 6.5 6.2 4.1 xxxxx xxxxx xxxxx xxxxx xxxxx  
Following: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxxx xxxxx xxxxx xxxxx xxxxx

Capacity Module:  
Conflict Vol: 1278 1217 441 1213 1213 748 752 xxxxx xxxxx xxxxx xxxxx  
Potent Cap.: 154 180 614 200 181 411 844 xxxxx xxxxx xxxxx xxxxx  
Move Cap.: 142 177 614 198 178 411 844 xxxxx xxxxx xxxxx xxxxx  
Volume/Cap: 0.00 0.00 0.00 0.06 0.00 0.07 0.01 xxxxx xxxxx xxxxx xxxxx

Level of Service Module:  
2Way95thQ: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.0 xxxxx xxxxx xxxxx xxxxx  
Control Del: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 9.3 xxxxx xxxxx xxxxx xxxxx  
LOS by Move: \* \* \* \* \*  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap.: xxxxx 0 xxxxx xxxxx 313 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx  
Shared Del: xxxxx xxxxx xxxxx 18.2 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx  
Shrd ConDel: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx  
Shared LOS: \* \* \* \* \* C \* \* \* \* \*  
ApproachDel: xxxxx \* 18.2 \* xxxxx \* xxxxx \*  
ApproachLOS: \* \* \* \* \* C \* \* \* \* \*

Note: Queue reported is the number of cars per lane.  
\*\*\*\*\*

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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Caldwell Avenue/Burke Street  
Average Delay (sec/veh): 2.6 Morat Case Level of Service: E( 37.0)

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0

Volume Module:  
Base Vol: 0 0 0 11 0 27 11 406 0 0 684 8  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:  
Critical Gap: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 XXXX XXXX 4.1 XXXX XXXX  
Followup: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 XXXX XXXX 2.2 XXXX XXXX

Capacity Module:  
Conflict Vol: 1252 1241 447 1248 1241 748 752 XXXX XXXX 452 XXXX XXXX  
Potential Cap: 148 174 609 149 174 411 844 XXXX XXXX 1093 XXXX XXXX

Level of Service Module:  
2way5thQ: XXXX XXXX XXXX XXXX XXXX XXXX 0.0 XXXX XXXX  
Control Del: XXXX XXXX XXXX XXXX XXXX XXXX 9.3 XXXX XXXX

Note: Queue reported is the number of cars per lane.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Caldwell Avenue/Burke Street  
Average Delay (sec/veh): 2.6 Morat Case Level of Service: E( 37.0)

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0

Volume Module:  
Base Vol: 0 0 0 11 0 27 11 406 0 0 684 8  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:  
Critical Gap: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 XXXX XXXX 4.1 XXXX XXXX  
Followup: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 XXXX XXXX 2.2 XXXX XXXX

Capacity Module:  
Conflict Vol: 1252 1241 447 1248 1241 748 752 XXXX XXXX 452 XXXX XXXX  
Potential Cap: 148 174 609 149 174 411 844 XXXX XXXX 1093 XXXX XXXX

Level of Service Module:  
2way5thQ: XXXX XXXX XXXX XXXX XXXX XXXX 0.0 XXXX XXXX  
Control Del: XXXX XXXX XXXX XXXX XXXX XXXX 9.3 XXXX XXXX

Note: Queue reported is the number of cars per lane.



Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report (HCM2000 Queue Method)  
2000 HCM Operations Method  
Base Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Green/Cycle:	0.00 0.00 0.00	0.25 0.00 0.25	0.11 0.63 0.00	0.00 0.00 0.52
ArrivalType:	3	3	3	3
ProdFactor:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Q1:	0.0 0.0 0.0	0.3 0.0 5.2	2.8 4.4 0.0	0.0 9.9 0.2
UpstreamVC:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
UpstreamAdj:	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00 0.00 0.00
EarlyArrAdj:	0.00 0.00 0.00	1.00 0.00 1.00	1.00 1.00 0.00	0.00 1.00 1.00
Q2:	0.0 0.0 0.0	0.0 0.0 1.1	1.1 0.4 0.0	0.0 1.1 0.0
HCM2KQueue:	0.0 0.0 0.0	0.4 0.0 6.3	3.8 4.8 0.0	0.0 11.0 0.2
70thFactor:	1.20 1.20 1.20	1.20 1.20 1.19	1.19 1.19 1.20	1.20 1.18 1.20
HCM2K70THQ:	0.0 0.0 0.0	0.5 0.0 7.5	4.6 5.7 0.0	0.0 13.0 0.3
85thFactor:	1.60 1.60 1.60	1.60 1.60 1.54	1.56 1.56 1.60	1.60 1.51 1.60
HCM2K85THQ:	0.0 0.0 0.0	0.6 0.0 9.8	6.0 7.4 0.0	0.0 16.6 0.4
90thFactor:	1.80 1.80 1.80	1.79 1.80 1.69	1.73 1.72 1.80	1.80 1.63 1.80
HCM2K90THQ:	0.0 0.0 0.0	0.7 0.0 10.7	6.6 8.2 0.0	0.0 18.0 0.4
95thFactor:	2.10 2.10 2.10	2.09 2.10 1.92	1.98 1.96 2.10	2.10 1.83 2.09
HCM2K95THQ:	0.0 0.0 0.0	0.8 0.0 12.2	7.6 9.3 0.0	0.0 20.1 0.5
98thFactor:	2.70 2.70 2.70	2.67 2.70 2.51	2.44 2.39 2.70	2.70 2.13 2.68
HCM2K98THQ:	0.0 0.0 0.0	1.0 0.0 14.7	9.4 11.4 0.0	0.0 23.5 0.6

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Fuel Consumption and Emissions  
2000 HCM Operations Method  
Base Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Run Speed:	30 MPH	30 MPH	30 MPH	30 MPH
NumStops:	0.0 0.0 0.0	3.1 0.0 46.9	25.0 39.2 0.0	0.0 89.0 1.8
Name: Year 1995 composite fleet				
Fuel Consumption:	31.988 pounds			
Carbon Dioxide:	5.177 gallons			
Carbon Monoxide:	99.710 pounds			
Hydrocarbons:	7.606 pounds			
Nitrogen Oxides:	1.313 pounds			
Name: Year 2000 composite fleet				
Fuel Consumption:	31.958 pounds			
Carbon Dioxide:	5.177 gallons			
Carbon Monoxide:	99.710 pounds			
Hydrocarbons:	7.606 pounds			
Nitrogen Oxides:	1.313 pounds			

DISCLAIMER  
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and HAREFIX Network models and are not appropriate since they can account for the influence of the adjacent control measures and other system elements.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour - Phase I

Level of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 Caldwell Avenue/Don Madsen Way

Cycle (sec): 100 Critical Vol./Cap. (X): 0.556  
Loss time (sec): 12 Average Delay (sec/veh): 20.5  
Optimal Cycle: 45 Level of Service: C

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected  
Rights: Include Include Include Include  
Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0  
Y+R: 0 0 0 0 1 0 0 0 1 0 1 0 0 0 0 1 0 1  
Lanes: 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:  
Base Vol: 0 0 0 15 0 15 0 199 97 320 0 0 493 14  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 15 0 15 0 199 97 320 0 0 493 14  
Added Vol: 0 0 0 0 0 0 4 16 15 0 0 0 0 4  
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0  
Initial Fut: 0 0 0 15 0 203 113 335 0 0 497 14  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 0 0 0 16 0 221 123 364 0 0 540 15  
Reduced Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0  
Reduced Vol: 0 0 0 16 0 221 123 364 0 0 540 15  
PEE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Volume: 0 0 0 16 0 221 123 364 0 0 540 15

Saturation Flow Module:  
Sat/Name: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900  
Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Adjustment: 0.00 0.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Lanes: 0 0 0 1805 0 1615 1805 1900 0 0 1900 1615  
Final Sat.: 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Capacity Analysis Module:  
Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.14 0.07 0.19 0.00 0.00 0.28 0.01  
Crit Moves: \*\*\*\*\*  
Green/Cycle: 0.00 0.00 0.00 0.25 0.00 0.25 0.12 0.63 0.00 0.00 0.51 0.51  
Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.56 0.30 0.00 0.00 0.00 0.56 0.02  
Delay/Veh: 0.0 0.0 0.0 28.7 0.0 34.7 44.4 8.4 0.0 0.0 17.4 12.0  
User Delay: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
AdjDel/Veh: 0.0 0.0 0.0 28.7 0.0 34.7 44.4 8.4 0.0 0.0 17.4 12.0  
LOS by Move: A A C A C D A A A B B  
HCMZKAVG: 0 0 0 0 0 0 7 4 5 0 0 11

Note: Queue reported is the number of cars per lane.

Traffic 8.0.0715 (c) 2008 Dowling Assoc. Licensed to OMBI-MEANS, VISALIA, CA

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report

2000 HCM Operations Method  
Future Volume Alternative

Intersection #2 Caldwell Avenue/Don Madsen Way

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected  
Rights: Include Include Include Include  
Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0  
Y+R: 0 0 0 0 1 0 0 0 1 0 1 0 0 0 0 1 0 1  
Lanes: 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 1 0 1

Volume Module:  
Base Vol: 0 0 0 15 0 15 0 199 97 320 0 0 493 14  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 15 0 15 0 199 97 320 0 0 493 14  
Added Vol: 0 0 0 0 0 0 4 16 15 0 0 0 0 4  
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0  
Initial Fut: 0 0 0 15 0 203 113 335 0 0 497 14  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 0 0 0 16 0 221 123 364 0 0 540 15  
Reduced Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0  
Reduced Vol: 0 0 0 16 0 221 123 364 0 0 540 15  
PEE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Volume: 0 0 0 16 0 221 123 364 0 0 540 15

Saturation Flow Module:  
Sat/Name: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900  
Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Adjustment: 0.00 0.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Lanes: 0 0 0 1805 0 1615 1805 1900 0 0 1900 1615  
Final Sat.: 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Capacity Analysis Module:  
Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.14 0.07 0.19 0.00 0.00 0.28 0.01  
Crit Moves: \*\*\*\*\*  
Green/Cycle: 0.00 0.00 0.00 0.25 0.00 0.25 0.12 0.63 0.00 0.00 0.51 0.51  
Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.56 0.30 0.00 0.00 0.00 0.56 0.02  
Delay/Veh: 0.0 0.0 0.0 28.7 0.0 34.7 44.4 8.4 0.0 0.0 17.4 12.0  
User Delay: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
AdjDel/Veh: 0.0 0.0 0.0 28.7 0.0 34.7 44.4 8.4 0.0 0.0 17.4 12.0  
LOS by Move: A A C A C D A A A B B  
HCMZKAVG: 0 0 0 0 0 0 7 4 5 0 0 11

Note: Queue reported is the number of cars per lane.

Traffic 8.0.0715 (c) 2008 Dowling Assoc. Licensed to OMBI-MEANS, VISALIA, CA

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report (HCM2000 Queue Method)  
2000 HCM Operations Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Green/Cycle: 0.00 0.00 0.00 0.25 0.00 0.25 0.12 0.63 0.00 0.00 0.51 0.51  
ArrivalType: 3 3 3 3  
ProgFactor: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Q1: 0.0 0.0 0.0 0.3 0.0 5.4 3.2 4.6 0.0 0.0 10.2 0.2  
QpstreamVC: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
UpstreamAdj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
EarlyStartAdj: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 0.00 0.00 1.00 1.00  
Q2: 0.0 0.0 0.0 0.0 0.0 1.2 1.1 0.4 0.0 0.0 1.2 0.0  
HCM2000Queue: 0.0 0.0 0.0 0.4 0.0 6.5 4.4 5.0 0.0 0.0 11.5 0.2  
\*\*\*\*\*  
70thFactor: 1.20 1.20 1.20 1.20 1.19 1.19 1.20 1.20 1.20 1.18 1.20  
HCM2070thQ: 0.0 0.0 0.0 0.5 0.0 7.7 5.2 6.0 0.0 0.0 13.5 0.3  
\*\*\*\*\*  
85thFactor: 1.60 1.60 1.60 1.60 1.60 1.60 1.56 1.55 1.60 1.60 1.50 1.60  
HCM2085thQ: 0.0 0.0 0.0 0.6 0.0 10.1 6.8 7.8 0.0 0.0 17.2 0.4  
\*\*\*\*\*  
90thFactor: 1.80 1.80 1.80 1.79 1.80 1.69 1.72 1.71 1.80 1.80 1.63 1.80  
HCM2090thQ: 0.0 0.0 0.0 0.7 0.0 11.0 7.5 8.6 0.0 0.0 18.6 0.4  
\*\*\*\*\*  
95thFactor: 2.10 2.10 2.10 2.09 2.10 1.92 1.97 1.95 2.10 2.10 1.82 2.09  
HCM2095thQ: 0.0 0.0 0.0 0.8 0.0 12.5 8.6 9.8 0.0 0.0 20.8 0.5  
\*\*\*\*\*  
98thFactor: 2.70 2.70 2.70 2.67 2.70 2.30 2.42 2.38 2.70 2.70 2.11 2.68  
HCM2098thQ: 0.0 0.0 0.0 1.0 0.0 15.1 10.5 11.9 0.0 0.0 24.2 0.6

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report (HCM2000 Queue Method)  
2000 HCM Operations Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Green/Cycle: 0.0 0.0 0.0 3.1 0.0 48.2 28.9 41.2 0.0 0.0 92.2 1.9  
ArrivalType: 3 3 3 3  
ProgFactor: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Q1: 0.0 0.0 0.0 0.3 0.0 5.4 3.2 4.6 0.0 0.0 10.2 0.2  
QpstreamVC: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
UpstreamAdj: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 0.00 0.00 1.00 1.00  
EarlyStartAdj: 0.00 0.00 0.00 1.2 1.1 0.4 0.0 0.0 1.2 0.0  
Q2: 0.0 0.0 0.0 0.4 0.0 6.5 4.4 5.0 0.0 0.0 11.5 0.2  
HCM2000Queue: 0.0 0.0 0.0 0.4 0.0 6.5 4.4 5.0 0.0 0.0 11.5 0.2  
\*\*\*\*\*  
70thFactor: 1.20 1.20 1.20 1.20 1.19 1.19 1.20 1.20 1.20 1.18 1.20  
HCM2070thQ: 0.0 0.0 0.0 0.5 0.0 7.7 5.2 6.0 0.0 0.0 13.5 0.3  
\*\*\*\*\*  
85thFactor: 1.60 1.60 1.60 1.60 1.60 1.60 1.56 1.55 1.60 1.60 1.50 1.60  
HCM2085thQ: 0.0 0.0 0.0 0.6 0.0 10.1 6.8 7.8 0.0 0.0 17.2 0.4  
\*\*\*\*\*  
90thFactor: 1.80 1.80 1.80 1.79 1.80 1.69 1.72 1.71 1.80 1.80 1.63 1.80  
HCM2090thQ: 0.0 0.0 0.0 0.7 0.0 11.0 7.5 8.6 0.0 0.0 18.6 0.4  
\*\*\*\*\*  
95thFactor: 2.10 2.10 2.10 2.09 2.10 1.92 1.97 1.95 2.10 2.10 1.82 2.09  
HCM2095thQ: 0.0 0.0 0.0 0.8 0.0 12.5 8.6 9.8 0.0 0.0 20.8 0.5  
\*\*\*\*\*  
98thFactor: 2.70 2.70 2.70 2.67 2.70 2.30 2.42 2.38 2.70 2.70 2.11 2.68  
HCM2098thQ: 0.0 0.0 0.0 1.0 0.0 15.1 10.5 11.9 0.0 0.0 24.2 0.6

Diamond Oaks TJAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Caldwell Avenue/Edison Street  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0

Volume Module:  
Base Vol: 0 0 0 0 0 0 0 0 417 0 0 692 0  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 0 0 0 0 0 417 0 0 692 0  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Volume: 0 0 0 0 0 0 0 0 417 0 0 692 0  
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0  
FinalVolume: 0 0 0 0 0 0 0 0 417 0 0 692 0

Critical Gap Module:  
Critical Gap: 6.2 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
FollowUpTime: 3.3 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX

Capacity Module:  
Conflict Vol: 417 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Potential Cap: 640 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Move Cap: 540 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Volume/Cap: XXXX XXXX 0.00 XXXX XXXX XXXX XXXX XXXX XXXX

Level of Service Module:  
2WayStop: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Control Del: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
LOS By Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: XXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared Queue: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shrd Conpel: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared LOS: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Approach LOS: XXXXXX XXXXXX XXXXXX XXXXXX

Note: Queue reported is the number of cars per lane.

Diamond Oaks TJAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Onsignalized Method

Intersection #3 Caldwell Avenue/Edison Street  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0

Volume Module:  
Base Vol: 0 0 0 0 0 0 0 0 417 0 0 692 0  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 0 0 0 0 0 417 0 0 692 0  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Volume: 0 0 0 0 0 0 0 0 417 0 0 692 0  
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0  
FinalVolume: 0 0 0 0 0 0 0 0 417 0 0 692 0

Critical Gap Module:  
Critical Gap: 6.2 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
FollowUpTime: 3.3 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX

Capacity Module:  
Conflict Vol: 417 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Potential Cap: 640 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Move Cap: 540 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Volume/Cap: XXXX XXXX 0.00 XXXX XXXX XXXX XXXX XXXX XXXX

Level of Service Module:  
2WayStop: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Control Del: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
LOS By Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: XXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared Queue: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shrd Conpel: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared LOS: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Approach LOS: XXXXXX XXXXXX XXXXXX XXXXXX

Note: Queue reported is the number of cars per lane.



Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level Of Service Detailed Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Caldwell Avenue/Edison Street

Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [10.9]

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0

Volume Module:

Base Vol: 0 0 0 0 0 0 0 0 417 0 0 692 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Base: 0 0 0 0 0 0 0 0 417 0 0 692 0

Added Vol: 0 0 15 0 0 0 0 0 16 1 0 8 0

PasserbyVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Svt: 0 0 15 0 0 0 0 0 433 1 0 700 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 0 0 15 0 0 0 0 0 433 1 0 700 0

Reeduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Final Volume: 0 0 15 0 0 0 0 0 433 1 0 700 0

Critical Gap Module:

Critical Gap: xxxxx xxx 5.2 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Followup Gap: xxxxx xxx 3.3 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: xxxxx xxx 434 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Potent Cap: xxx xxx 627 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Move Cap: xxx xxx 627 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Volume/Cap: xxx xxx 0.02 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95th: xxx xxx 0.1 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Control Rel: xxx xxx 10.9 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: \* B \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shared Queue: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shrd Conbel: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shred LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Approach Del: 10.9 xxxxxx \* xxxxxx \* xxxxxx \*

Approach LOS: B \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Note: Queue reported is the number of cars per lane.

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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level Of Service Detailed Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Caldwell Avenue/Edison Street

Average Delay (sec/veh): 0.1 Worst Case Level of Service: B [10.9]

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0

Volume Module:

Base Vol: 0 0 0 0 0 0 0 0 417 0 0 692 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Base: 0 0 0 0 0 0 0 0 417 0 0 692 0

Added Vol: 0 0 15 0 0 0 0 0 16 1 0 8 0

PasserbyVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Svt: 0 0 15 0 0 0 0 0 433 1 0 700 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 0 0 15 0 0 0 0 0 433 1 0 700 0

Reeduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Final Volume: 0 0 15 0 0 0 0 0 433 1 0 700 0

Critical Gap Module:

Critical Gap: xxxxx xxx 5.2 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Followup Gap: xxxxx xxx 3.3 xxxxx xxx xxxxx xxxxx xxxxx xxxxx xxxxx

Capacity Module:

Conflict Vol: xxxxx xxx 434 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

Potent Cap: xxx xxx 627 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

Move Cap: xxx xxx 627 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

Volume/Cap: xxx xxx 0.02 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

Level Of Service Module:

2Way95th: xxx xxx 0.1 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

Control Rel: xxx xxx 10.9 xxxxx xxx xxxxx xxxxx xxxxx xxxxx

LOS by Move: \* B \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shared Queue: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shrd Conbel: xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx xxx

Shred LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Approach Del: 10.9 xxxxxx \* xxxxxx \* xxxxxx \*

Approach LOS: B \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Note: Queue reported is the number of cars per lane.

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Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour - Phase I

Level of Service Computation Report  
2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #4 Russell Ave/Burke St

Average Delay (sec/vch): 3.6 Worst Case Level of Service: A [ 9.3]

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 1

Volume Module:

Base Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Base: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Added Vol: 0 12 0 15 4 0 0 0 0 0 0 0 0 0 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 0 12 0 15 4 0 0 0 0 0 0 0 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 0 12 0 15 4 0 0 0 0 0 0 0 0 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 0 12 0 15 4 0 0 0 0 0 0 0 0 0 0 0 0

Critical Gap Module:

Critical Gap: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

FollowupTime: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Capacity Module:

Conflict Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Potent Cap.: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Move Cap.: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Volume/Cap: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

Level of Service Module:

2PM55THQ: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Control Del: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

LOS by Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Shared Queue: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Shared Conbel: 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0

Shared LOS: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

ApproachDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

ApproachLOS: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Note: Queue reported is the number of cars per lane.

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Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour - Phase I

Level of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #4 Russell Ave/Burke St

Average Delay (sec/vch): 0.0 Worst Case Level of Service: [ 0.0]

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 1

Volume Module:

Base Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Growth Adj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

Initial Base: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

User Adj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

PHF Adj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

PHF Volume: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Critical Gap Module:

Critical Gap: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

FollowupTime: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Capacity Module:

Conflict Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Potent Cap.: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Move Cap.: 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

Volume/Cap: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

Level of Service Module:

2WAY55THQ: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Control Del: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

LOS by Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Shared Queue: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Shared Conbel: 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0

Shared LOS: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

ApproachDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

ApproachLOS: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

Note: Queue reported is the number of cars per lane.

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Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level Of Service Detailed Computation Report  
2000 HCM Unsignalized Method  
Picture Volume Alternative

\*\*\*\*\*  
Intersection #4 Russell Ave/Burke St  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
BvWVah: 0% 0% 0% 0%  
Grade: 0% 0% 0% 0%  
Peds/Hour: 0 0 0 0  
Pedestrian Walk Speed: 4.00 feet/sec  
LaneWidth: 12 feet 12 feet 12 feet 12 feet  
Time Period: 0.25 hour

Diamond Oaks TIAE  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level Of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

\*\*\*\*\*  
Intersection #5 Cameron Ave/Burke St  
\*\*\*\*\*  
Average Delay (sec/veh): 0.0 Worst Case Level Of Service: [ 0.0 ]  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Step Sign Step Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1  
Volume Module:  
Base Vol: 0  
Growth Adj: 0.00  
Initial Bse: 0  
User Adj: 0.00  
PHE Adj: 0.00  
PHE Volume: 0  
Reduct Vol: 0  
FinalVolume: 0  
Critical Gap Modules:  
Critical Gap: 0.0  
FollowupTm: 0.0  
Capacity Module:  
Conflict Vol: 0  
Potential Cap.: 0  
Move Cap.: 1  
Volume/Cap: 0.00  
Level Of Service Module:  
2way95thQ: 0.0  
Control Del: 0.0  
LOS By Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap.: 0  
SharedQueue: 0.0  
Shared Condel: 1.0  
Shared LOS: 0.0  
ApproachLOS: 0.0 0.0 0.0 0.0  
\*\*\*\*\*  
Note: Queue reported is the number of cars per lane.  
\*\*\*\*\*

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour + Phase I

Level Of Service Detailed Computation Report  
2000 HCM Unsignalized Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #5 Cameron Ave/Burke St  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
HevVeh: 0% 0% 0% 0%  
Grade: 0% 0% 0% 0%  
Pedestrian Walk Speed: 4.00 feet/sec  
LaneWidth: 12 feet 12 feet 12 feet 12 feet  
Time Period: 0.25 hour

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing AM Peak Hour Phase I

Level Of Service Detailed Computation Report  
2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*  
Intersection #5 Cameron Ave/Burke St  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
HevVeh: 0% 0% 0% 0%  
Grade: 0% 0% 0% 0%  
Pedestrian Walk Speed: 4.00 feet/sec  
LaneWidth: 12 feet 12 feet 12 feet 12 feet  
Time Period: 0.25 hour

Level of Service Module:  
2WayStg: 0.00 0.00 0.00 0.00  
Control Del: 0.00 0.00 0.00 0.00  
LOS by Move: A A A A  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap: 0.00 0.00 0.00 0.00  
Shared Queue: 0.00 0.00 0.00 0.00  
Shrd ConDel: 0.00 0.00 0.00 0.00  
Shared LOS: A A A A  
ApproachDel: 0.00 0.00 0.00 0.00  
ApproachLOS: A A A A  
\*\*\*\*\*  
Note: Queue reported is the number of cars per lane.  
\*\*\*\*\*

Critical Gap Module:  
Critical Gap: 6.5 6.2 6.4 6.4  
FollowThru: 4.0 3.3 3.5 3.5  
\*\*\*\*\*

Capacity Module:  
Conflict Vol: 12 0 0 0  
Patient Cap: 887 1091 1029 1029  
Move Cap: 887 1091 1029 1029  
Volume/Cap: 0.00 0.00 0.00 0.00  
\*\*\*\*\*

Level of Service Module:  
2WayStg: 0.00 0.00 0.00 0.00  
Control Del: 0.00 0.00 0.00 0.00  
LOS by Move: A A A A  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap: 0.00 0.00 0.00 0.00  
Shared Queue: 0.00 0.00 0.00 0.00  
Shrd ConDel: 0.00 0.00 0.00 0.00  
Shared LOS: A A A A  
ApproachDel: 0.00 0.00 0.00 0.00  
ApproachLOS: A A A A  
\*\*\*\*\*  
Note: Queue reported is the number of cars per lane.  
\*\*\*\*\*

Trip Generation Report

Forecast for PM Peak

Zone #	Subzone	Amount	Units	Rate		Trips		Trips Total
				In	Out	In	Out	
1	Zone 1	121.00	PM Peak	0.65	0.35	79	42	121 100.0
TOTAL								121 100.0

Scenario Report

PM Phase I

Command: Default Command  
 Volume: PM Peak  
 Geometry: Phase I  
 Impact Fee: Default Impact Fee  
 Trip Generation: PM Peak  
 Trip Distribution: Existing Plus Phase I  
 Paths: Default Path  
 Routes: Default Route  
 Configuration: Default Configuration

Trip Distribution Report

Zone	Percent Of Trips Default									
	1	2	3	4	6	7	8	10	11	
1	10.0	8.0	22.0	20.0	5.0	5.0	5.0	20.0	5.0	

Turning Movement Report

Volume	Northbound		Southbound		Eastbound		Westbound		Total
	Left	Thru Right	Left	Thru Right	Left	Thru Right	Left	Thru Right	
<b>#1 Caldwell Avenue/Burke Street</b>									
Base	0	0	4	0	26	31	945	0	0
Added	21	3	9	0	6	0	4	36	33
Total	21	3	13	0	32	31	949	36	33
<b>#2 Caldwell Avenue/Ben Maddox Way</b>									
Base	0	0	0	12	0	253	691	0	0
Added	0	0	0	0	17	9	8	0	16
Total	0	0	0	12	17	262	699	0	16
<b>#3 Caldwell Avenue/Edison Street</b>									
Base	0	0	0	0	0	0	949	0	0
Added	0	0	9	0	0	0	9	4	33
Total	0	0	9	0	0	0	958	4	33
<b>#4 Russell Ave/Burke St</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	7	0	60	15	0	0	0	82
Total	0	7	0	60	15	0	0	0	82
<b>#5 Cameron Ave/Burke St</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	15	0	0	0	0	15
Total	0	0	0	15	0	0	0	0	15
<b>#6 Russell Ave/Ben Maddox Way</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0
<b>#7 Cameron Ave/Ben Maddox Way</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0
<b>#8</b>									
Base	0	0	0	0	0	0	0	0	0
Added	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0

Diamond Oaks TIR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I

Impact Analysis Report  
 Level Of Service

Intersection	Base Del/ V/ LOS Veh C	Future Del/ V/ LOS Veh C	Change In
# 1 Caldwell Avenue/Burke Street	E 39.6 0.118	F 613.8 1.375	+578.221 D/V
# 2 Caldwell Avenue/Ben Maddox Way	C 31.4 0.878	C 34.6 0.907	+ 3.264 D/V
# 3 Caldwell Avenue/Edison Street	A 0.0 0.000	C 18.3 0.035	+18.277 D/V
# 4 Russell Ave/Burke St	0.0 0.000	A 9.2 0.059	+ 9.171 D/V
# 5 Cameron Ave/Burke St	0.0 0.000	A 8.6 0.015	+ 8.552 D/V

Diamond Oaks TIR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I

Signal Warrant Summary Report

Intersection	Base Met [Del / Yell]	Future Met [Del / Yell]
# 1 Caldwell Avenue/Burke Street	No / No	No / No
# 3 Caldwell Avenue/Edison Street	No / No	No / No
# 4 Russell Ave/Burke St	No / No	No / No
# 5 Cameron Ave/Burke St	No / No	No / No

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour - Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0

Initial Vol: 0 0 0 0 4 0 26 31 945 0 0 1059 4  
ApproachDel: xxxxxx 35.6 xxxxxx xxxxxx

Approach[southbound][lanes=1][control=Stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=0.3]  
Signal Warrant Rule #2: [approach volume=30]  
Signal Warrant Rule #3: [approach count=3][total volume=2069]

SUCCESS - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour - Phase I

Peak Hour Volume Signal Warrant Report [Urban]

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0

Initial Vol: 0 0 0 0 4 0 26 31 945 0 0 1059 4  
ApproachDel: xxxxxx 35.6 xxxxxx xxxxxx

Approach[southbound][lanes=1][control=Stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=0.3]  
Signal Warrant Rule #2: [approach volume=30]  
Signal Warrant Rule #3: [approach count=3][total volume=2069]

SUCCESS - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.



Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0  
Initial Vol: 21 3 9 4 6 26 31 949 36 33 1059 4  
ApproachVol: 613.8 102.7  
\*\*\*\*\*

Approach[northbound][lanes=1][control=stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=5.6]  
SUCCEED - Vehicle-hours greater than or equal to 4 for one lane approach.  
Signal Warrant Rule #2: [approach volume=33]  
FAIL - Approach volume less than 100 for one lane approach.  
Signal Warrant Rule #3: [approach count=4][total volume=2181]  
SUCCEED - Total volume greater than or equal to 800 for intersection  
with four or more approaches.

Approach[southbound][lanes=1][control=stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=1.0]  
FAIL - Vehicle-hours less than 4 for one lane approach.  
Signal Warrant Rule #2: [approach volume=36]  
FAIL - Approach volume less than 100 for one lane approach.  
Signal Warrant Rule #3: [approach count=4][total volume=2181]  
SUCCEED - Total volume greater than or equal to 800 for intersection  
with four or more approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an  
"indicator" of the likelihood of an unsignalized intersection warranting  
a traffic signal in the future. Intersections that exceed this warrant  
are probably more likely to meet one or more of the other volume based  
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace  
a rigorous and complete traffic signal warrant analysis by the responsible  
jurisdiction. Consideration of the other signal warrants, which is beyond  
the scope of this software, may yield different results.

Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0  
Initial Vol: 21 3 9 4 6 26 31 949 36 33 1059 4  
ApproachVol: 613.8 102.7  
\*\*\*\*\*

Approach[northbound][lanes=1][control=stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=5.6]  
SUCCEED - Vehicle-hours greater than or equal to 4 for one lane approach.  
Signal Warrant Rule #2: [approach volume=33]  
FAIL - Approach volume less than 100 for one lane approach.  
Signal Warrant Rule #3: [approach count=4][total volume=2181]  
SUCCEED - Total volume greater than or equal to 800 for intersection  
with four or more approaches.

Approach[southbound][lanes=1][control=stop Sign]  
Signal Warrant Rule #1: [vehicle-hours=1.0]  
FAIL - Vehicle-hours less than 4 for one lane approach.  
Signal Warrant Rule #2: [approach volume=36]  
FAIL - Approach volume less than 100 for one lane approach.  
Signal Warrant Rule #3: [approach count=4][total volume=2181]  
SUCCEED - Total volume greater than or equal to 800 for intersection  
with four or more approaches.

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an  
"indicator" of the likelihood of an unsignalized intersection warranting  
a traffic signal in the future. Intersections that exceed this warrant  
are probably more likely to meet one or more of the other volume based  
signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace  
a rigorous and complete traffic signal warrant analysis by the responsible  
jurisdiction. Consideration of the other signal warrants, which is beyond  
the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 0 0 0
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 949 0	0 1050 0
Approachbei:	XXXXXX	XXXXXX	XXXXXX	XXXXXX

Major Street Volume: 1999  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: 35 [Less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 1 0 0
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 949 0	0 1050 0
Approachbei:	XXXXXX	XXXXXX	XXXXXX	XXXXXX

Major Street Volume: 1999  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: 35 [Less than minimum of 100]

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 1 0 0
Initial Vol:	0 0 0 9	0 0 0 0 0	0 958	4 0 1083 0
Approach Vol:	18.3	xxxxxx	xxxxxx	xxxxxx

Approach(northbound){lanes=1}{control=stop Sign}  
 Signal Warrant Rule #1: {vehicle-hours=0.0}  
 FAIL - Vehicle-hours less than 4 for one lane approach.  
 Signal Warrant Rule #2: {approach volume=9}  
 FAIL - Approach volume less than 100 for one lane approach.  
 Signal Warrant Rule #3: {approach count=3}{total volume=2054}  
 SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 0 1	0 0 0 0 0	0 0 0 1 0	0 0 1 0 0
Initial Vol:	0 0 0 9	0 0 0 0 0	0 958	4 0 1083 0
Approach Vol:	18.3	xxxxxx	xxxxxx	xxxxxx

Approach(northbound){lanes=1}{control=stop Sign}  
 Signal Warrant Rule #1: {vehicle-hours=0.0}  
 FAIL - Vehicle-hours less than 4 for one lane approach.  
 Signal Warrant Rule #2: {approach volume=9}  
 FAIL - Approach volume less than 100 for one lane approach.  
 Signal Warrant Rule #3: {approach count=3}{total volume=2054}  
 SUCCEEDED - Total volume greater than or equal to 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER  
 This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #4 Russell Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0  
Initial Vol: 0  
ApproachDel: 0.0 0.0 0.0 0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: 1mf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Volume Signal Warrant Report (Urban)

Intersection #4 Russell Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0  
Initial Vol: 0  
ApproachDel: 0.0 0.0 0.0 0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: 1mf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIAR  
55-2434-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #4 Russell Ave/Burke St

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 1

Initial Vol: 0 7 0 60 15 0 0 0 0 0 0 0 0 0 0 0 27

ApproachDel: 5.2 8.3 xxxxxx

Approach[northbound][lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0]

FAIL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant Rule #2: [approach volume=7]

FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #3: [approach count=3][total volume=109]

FAIL - Total volume less than 650 for intersection with less than four approaches.

Approach[southbound][lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0.2]

FAIL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant Rule #2: [approach volume=75]

FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #3: [approach count=3][total volume=109]

FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIAR  
55-2434-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #4 Russell Ave/Burke St

Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 1

Initial Vol: 0 7 0 60 15 0 0 0 0 0 0 0 0 0 0 0 27

ApproachDel: 5.2 8.3 xxxxxx

Approach[northbound][lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0]

FAIL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant Rule #2: [approach volume=7]

FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #3: [approach count=3][total volume=109]

FAIL - Total volume less than 650 for intersection with less than four approaches.

Approach[southbound][lanes=1][control=stop sign]

Signal Warrant Rule #1: [vehicle-hours=0.2]

FAIL - Vehicle-hours less than 4 for one lane approach.

Signal Warrant Rule #2: [approach volume=75]

FAIL - Approach volume less than 100 for one lane approach.

Signal Warrant Rule #3: [approach count=3][total volume=109]

FAIL - Total volume less than 650 for intersection with less than four approaches.

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #5 Cameron Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 1 0	1 0 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: +Inf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
The peak hour warrant analysis in this report is not intended to replace a rigorous and complete traffic signal warrant analysis by the responsible jurisdiction. Consideration of the other signal warrants, which is beyond the scope of this software, may yield different results.

Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report

Intersection #5 Cameron Ave/Burke St

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Stop Sign	Stop Sign	Uncontrolled	Uncontrolled
Lanes:	0 0 0 1 0	1 0 0 0 0	0 0 0 0 0	0 0 0 0 1
Initial Vol:	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0
ApproachDel:	0.0	0.0	0.0	0.0

Major Street Volume: 0  
Minor Approach Volume: 0  
Minor Approach Volume Threshold: +Inf

SIGNAL WARRANT DISCLAIMER

This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).  
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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
Intersection #5 Cameron Ave/Burke St  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R L T R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 0 1 0 1 0 0 0 0 0 0 0 0 0 0 1  
Initial Vol: 0 0 0 0 15 0 0 0 0 0 0 0 0 0 0 0 7  
ApproachDel: xxxxxx 8.6 xxxxxx xxxxxx  
Major Street Volume: 7  
Minor Approach Volume: 15  
Minor Approach Volume Threshold: 1543

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Diamond Oaks TIR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Peak Hour Delay Signal Warrant Report  
Intersection #5 Cameron Ave/Burke St  
Future Volume Alternative: Peak Hour Warrant NOT Met

Approach: North Bound South Bound East Bound West Bound  
Movement: L T R L T R L T R L T R L T R  
Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Lanes: 0 0 0 1 0 1 0 0 0 0 0 0 0 0 0 0 1  
Initial Vol: 0 0 0 0 15 0 0 0 0 0 0 0 0 0 0 0 7  
ApproachDel: xxxxxx 8.6 xxxxxx xxxxxx  
Major Street Volume: 7  
Minor Approach Volume: 15  
Minor Approach Volume Threshold: 1543

SIGNAL WARRANT DISCLAIMER  
This peak hour signal warrant analysis should be considered solely as an "indicator" of the likelihood of an unsignalized intersection warranting a traffic signal in the future. Intersections that exceed this warrant are probably more likely to meet one or more of the other volume based signal warrant (such as the 4-hour or 8-hour warrants).

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Level of Service Computation Report  
 2000 HCM Unsignalized Method  
 Base Volume Alternative

Intersection #1 Caldwell Avenue/Burke Street  
 Approach: North Bound South Bound East Bound West Bound  
 Movement: L - T - R L - T - R L - T - R L - T - R  
 Rev/Veh: 3% 3% 5% 5%  
 Grade: 0% 0% 0% 0%  
 Pedestrian Walk Speed: 4.00 feet/sec 0  
 Lane Width: 12 feet 12 feet 12 feet 12 feet  
 Time Period: 0.25 hour  
 Upstream Signals:  
 Link Index: #6  
 Dist(miles): 0.00  
 Speed (mph): #2  
 Signal Index:  
 Cycle Time: 0 secs  
 Init Volume:  
 Saturation:  
 Arrival Type:  
 G/C: 0.00 0.00  
 P: 0.00 0.00  
 Sg1: 0.00 0.00  
 Sg2: 0.00 0.00  
 alpha: 0.00 0.00  
 beta: 0.00 0.00  
 tau (secs): 0.00 0.00  
 E: 0.00 0.00  
 vcmx: 0 0  
 vcm: 0 0  
 tp: 0.0 0.0  
 P: Computation 3: Platoon Event Periods  
 Pcm/psubc: 0.060/0.000/0.000/0.000/0.000/0.000  
 Computation 4: Conflicting Flows During Each Unblocked Period  
 InitCtrlVol: 2250 1027 2248 2248 1153 1155 1155 1155  
 AdjCtrlVol: 2262 2250 1027 2248 2248 1153 1155 1155  
 UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 ConflictVol: 2250 1027 2248 2248 1153 1155 1155 1155  
 Computation 5: Capacity for Subject Movement During Unblocked Period  
 InitRetCap: 29 41 283 46 41 239 594 594  
 UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 Potent Cap: 29 41 283 46 41 239 594 594

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Level of Service Computation Report  
 2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Caldwell Avenue/Burke Street  
 Approach: North Bound South Bound East Bound West Bound  
 Movement: L - T - R L - T - R L - T - R L - T - R  
 Rev/Veh: 3% 3% 5% 5%  
 Grade: 0% 0% 0% 0%  
 Pedestrian Walk Speed: 4.00 feet/sec 0  
 Lane Width: 12 feet 12 feet 12 feet 12 feet  
 Time Period: 0.25 hour  
 Upstream Signals:  
 Link Index: #6  
 Dist(miles): 0.00  
 Speed (mph): #2  
 Signal Index:  
 Cycle Time: 0 secs  
 Init Volume:  
 Saturation:  
 Arrival Type:  
 G/C: 0.00 0.00  
 P: 0.00 0.00  
 Sg1: 0.00 0.00  
 Sg2: 0.00 0.00  
 alpha: 0.00 0.00  
 beta: 0.00 0.00  
 tau (secs): 0.00 0.00  
 E: 0.00 0.00  
 vcmx: 0 0  
 vcm: 0 0  
 tp: 0.0 0.0  
 P: Computation 3: Platoon Event Periods  
 Pcm/psubc: 0.060/0.000/0.000/0.000/0.000/0.000  
 Computation 4: Conflicting Flows During Each Unblocked Period  
 InitCtrlVol: 2250 1027 2248 2248 1153 1155 1155 1155  
 AdjCtrlVol: 2262 2250 1027 2248 2248 1153 1155 1155  
 UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 ConflictVol: 2250 1027 2248 2248 1153 1155 1155 1155  
 Computation 5: Capacity for Subject Movement During Unblocked Period  
 InitRetCap: 29 41 283 46 41 239 594 594  
 UpstreamAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 Potent Cap: 29 41 283 46 41 239 594 594

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Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Unsignalized Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Level: 3% 3% 5% 5%  
Grade: 0% 0% 0% 0%  
Pedestrian Walk Speed: 4.00 feet/sec 0 0 0  
Lane Width: 12 feet 12 feet 12 feet 12 feet

Time Period: 0.25 hour  
Upstream Signals:  
Link Index: #6  
Dist (miles): 0.00  
Speed (mph): #2  
Signal Index: 0 0 0 0  
Cycle Time: 0 0 0 0  
Init Volume: 0 0 0 0  
Saturation: 0.92 0.92 0.92 0.92  
Arrival Type: 3 10 4 7 28 34 1032 39 36 1151 4

\*\*\* Computation 1: Time for Queue to Clear at Each Upstream Intersection  
P: 0.000 0.000  
gq1: 0.00 0.00  
gq2: 0.00 0.00  
gq: 0.00 0.00  
\*\*\* Computation 2: Time Intersection Blocked Because of Upstream Platoons  
alpha: 0.000  
beta: 0.000  
Ls (secs): 0.000  
F: 0.000 0.000  
vcmax: 0 0  
vcg: 0 0  
vcmin: 0 0  
cp: 0.0 0.0

\*\*\* Computation 3: Platoon Event Periods  
beta/psdco: 0.000/0.000/unconstrained  
\*\*\* Computation 4: Conflicting Flows During Each Unblocked Period  
InitCnfVol:2361 2396 1051 2350 2163 1153 1155 XXXXX XXXXX 1071 XXXX XXXX  
AdjCnfVol: 2361 2396 1051 2350 2163 1153 1155 XXXXX XXXXX 1071 XXXX XXXX  
UpstreamAdj:1.00 1.000 1.000 1.000 1.000 1.000 1.000 XXXX XXXX  
ConflictVol:2361 2396 1051 2390 2368 1153 1155 XXXXX XXXXX 1071 XXXX XXXX

\*\*\* Computation 5: Capacity for Subject Movement During Unblocked Periods  
InitPotCap: 24 36 274 25 35 239 594 XXXXX XXXX 640 XXXX XXXX  
UpstreamAdj:1.00 1.000 1.000 1.000 1.000 1.000 XXXX XXXX  
Potent Cap: 24 36 274 25 35 239 594 XXXXX XXXX 640 XXXX XXXX

Diamond Oaks TIAF  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Computation Report  
2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*  
Intersection #1 Caldwell Avenue/Burke Street  
\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 1 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0

Volume Module:  
Base Vol: 0 0 0 4 0 26 31 945 0 0 1059 4  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Base: 0 0 0 4 0 26 31 945 0 0 1059 4  
Added Vol: 21 3 9 0 6 0 0 4 36 33 0 0  
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0  
Initial Fut: 21 3 9 4 6 26 31 949 36 33 1059 4  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 23 3 10 4 7 28 34 1032 39 36 1151 4  
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0  
Final Volume: 23 3 10 4 7 28 34 1032 39 36 1151 4

Critical Gap Module:  
Critical Gap: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 XXXX XXXX 4.1 XXXX XXXX  
Critical Gap: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 XXXX XXXX 2.2 XXXX XXXX  
Followup Gap: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 XXXX XXXX 2.2 XXXX XXXX

Capacity Module:  
Critical Vol: 2361 2396 1051 2350 2163 1153 1155 XXXX XXXX 1071 XXXX XXXX  
Potent Cap: 24 36 274 25 35 239 594 XXXX XXXX 640 XXXX XXXX  
Move Cap: 17 32 274 20 31 239 594 XXXX XXXX 640 XXXX XXXX  
Volume/Cap: 1.38 0.10 0.04 0.21 0.21 0.12 0.06 XXXX XXXX 0.06 XXXX XXXX

Level of Service Module:  
2WaySBQ: XXXX XXXX XXXX XXXX XXXX 0.2 XXXX XXXX  
Control Del:XXXX XXXX XXXX XXXX XXXX 11.4 XXXX XXXX 11.0 XXXX XXXX  
LOS by Move: \* \* \* \* \* B \* \* \* \* \* B \* \* \* \* \*  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap: XXXX 24 XXXXX XXXX 72 XXXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared Queue:XXXX 4.5 XXXXX XXXX 2.3 XXXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shrd Condel:XXXX 614 XXXXX XXXX 103 XXXXX XXXX XXXX XXXX XXXX XXXX  
Shared LOS: \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \*  
ApproachDel: 613.8 102.7 XXXXXX  
ApproachLOS: \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \* F \* \* \* \* \*

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Caldwell Avenue/Ben Maddox Way  
Base Volume Alternative

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected  
Rights: Include Include Include Include  
Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0  
Y+R: 0 0 0 0 1 0 0 1 1 0 1 0 0 1 0 1  
Lanes: 0 0 0 0 1 0 0 1 1 0 1 0 0 1 0 1

Volume Module:  
Base Vol: 0 0 0 12 0 253 258 691 0 0 781 8  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 12 0 253 258 691 0 0 781 8  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 0 0 0 13 0 275 280 751 0 0 849 9  
Reduced Vol: 0 0 0 0 0 0 0 0 0 0 0 0  
PCB Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Volume: 0 0 0 13 0 275 280 751 0 0 849 9

Saturation Flow Module:  
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900  
Reluctant: 1.00 1.00 1.00 0.95 1.00 0.85 0.95 1.00 1.00 1.00 0.85  
Lanes: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Sat.: 0 0 0 1805 0 1615 1805 1900 0 0 1900 1615

Capacity Analysis Module:  
Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.17 0.16 0.40 0.00 0.00 0.45 0.01  
Crit Moves: 0.00 0.00 0.00 0.19 0.00 0.19 0.18 0.69 0.00 0.00 0.51 0.01  
Green/Cycle: 0.00 0.00 0.00 0.04 0.00 0.88 0.88 0.58 0.00 0.00 0.58 0.01  
Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.88 0.88 0.58 0.00 0.00 0.58 0.01  
Delay/Veh: 0.0 0.0 0.0 32.8 0.0 62.5 63.1 8.8 0.0 0.0 31.0 12.1  
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
AdjDel/Veh: 0.0 0.0 0.0 32.8 0.0 62.5 63.1 8.8 0.0 0.0 31.0 12.1  
LOS by Move: A A A C A E A A A C B  
IJC2RWVQ: 0 0 0 0 0 11 12 12 0 0 26 0  
Note: Queue reported is the number of cars per lane.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Caldwell Avenue/Ben Maddox Way  
Base Volume Alternative

Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected  
Rights: Include Include Include Include  
Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0  
Y+R: 0 0 0 0 1 0 0 1 1 0 1 0 0 1 0 1  
Lanes: 0 0 0 0 1 0 0 1 1 0 1 0 0 1 0 1

Volume Module:  
Base Vol: 0 0 0 12 0 253 258 691 0 0 781 8  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 12 0 253 258 691 0 0 781 8  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHF Volume: 0 0 0 13 0 275 280 751 0 0 849 9  
Reduced Vol: 0 0 0 0 0 0 0 0 0 0 0 0  
PCB Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Volume: 0 0 0 13 0 275 280 751 0 0 849 9

Saturation Flow Module:  
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900  
Reluctant: 1.00 1.00 1.00 0.95 1.00 0.85 0.95 1.00 1.00 1.00 0.85  
Lanes: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00  
Final Sat.: 0 0 0 1805 0 1615 1805 1900 0 0 1900 1615

Capacity Analysis Module:  
Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.17 0.16 0.40 0.00 0.00 0.45 0.01  
Crit Moves: 0.00 0.00 0.00 0.19 0.00 0.19 0.18 0.69 0.00 0.00 0.51 0.01  
Green/Cycle: 0.00 0.00 0.00 0.04 0.00 0.88 0.88 0.58 0.00 0.00 0.58 0.01  
Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.88 0.88 0.58 0.00 0.00 0.58 0.01  
Delay/Veh: 0.0 0.0 0.0 32.8 0.0 62.5 63.1 8.8 0.0 0.0 31.0 12.1  
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
AdjDel/Veh: 0.0 0.0 0.0 32.8 0.0 62.5 63.1 8.8 0.0 0.0 31.0 12.1  
LOS by Move: A A A C A E A A A C B  
IJC2RWVQ: 0 0 0 0 0 11 12 12 0 0 26 0  
Note: Queue reported is the number of cars per lane.

Diamond Oaks TIAR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I  
 Fuel Consumption and Emissions  
 2000 HCM Operations Method  
 Base Volume Alternative

\*\*\*\*\*  
 Intersection #2 Caldwell Avenue/Ben Maddox Way  
 Approach: North Bound South Bound East Bound West Bound  
 Movement: L - T - R L - T - R L - T - R L - T - R

Run Speed:	0.0	0.0	0.0	2.6	0.0	66.8	68.3	87.5	0.0	0.0	188	1.1
NumOfStops:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Name: Year 1995 composite fleet  
 Fuel Consumption: 74.365 gallons  
 Carbon Dioxide: 232.020 pounds  
 Carbon Monoxide: 18.455 pounds  
 Hydrocarbons: 3.423 pounds  
 Nitrogen Oxides: 0.832 pounds

Name: Year 2000 composite fleet  
 Fuel Consumption: 74.365 gallons  
 Carbon Dioxide: 232.020 pounds  
 Carbon Monoxide: 18.455 pounds  
 Hydrocarbons: 3.423 pounds  
 Nitrogen Oxides: 0.832 pounds

DISCLAIMER  
 The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Diamond Oaks TIAR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I  
 Level of Service Detailed Computation Report (HCM2000 Queue Method)  
 2000 HCM Operations Method  
 Base Volume Alternative

\*\*\*\*\*  
 Intersection #2 Caldwell Avenue/Ben Maddox Way  
 Approach: North Bound South Bound East Bound West Bound  
 Movement: L - T - R L - T - R L - T - R L - T - R

Green/Cycle:	0.00	0.00	0.00	0.19	0.00	0.19	0.18	0.69	0.00	0.00	0.51	0.51
ArrivalType:	3	3	3	3	3	3	3	3	3	3	3	3
ProdFactor:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Q1:	0.0	0.0	0.0	0.3	0.0	0.3	0.0	7.4	7.6	10.8	0.0	0.0
UpstreamVC:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
UpstreamAdj:	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
EarlyArrAdj:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.00	1.00
Q2:	0.0	0.0	0.0	0.0	0.0	3.9	4.0	1.3	0.0	0.0	5.3	0.0
HCM2KQueue:	0.0	0.0	0.0	0.3	0.0	11.4	11.6	12.2	0.0	0.0	26.2	0.1

70thFactor:	1.20	1.20	1.20	1.18	1.17	1.17	1.20	1.20	1.15	1.20	1.20	1.20
HCM2K70thQ:	0.0	0.0	0.0	0.4	0.0	13.4	13.6	14.3	0.0	0.0	30.2	0.2
85thFactor:	1.60	1.60	1.60	1.51	1.50	1.50	1.60	1.60	1.43	1.60	1.60	1.60
HCM2K85thQ:	0.0	0.0	0.0	0.5	0.0	17.1	17.4	18.3	0.0	0.0	37.4	0.2

90thFactor:	1.80	1.80	1.80	1.79	1.80	1.63	1.62	1.62	1.80	1.80	1.51	1.80
HCM2K90thQ:	0.0	0.0	0.0	0.6	0.0	18.5	18.8	19.7	0.0	0.0	39.5	0.2
95thFactor:	2.10	2.10	2.10	2.09	2.10	1.82	1.82	1.81	2.10	2.10	1.64	2.10
HCM2K95thQ:	0.0	0.0	0.0	0.7	0.0	20.7	21.0	22.0	0.0	0.0	43.0	0.3

98thFactor:	2.70	2.70	2.70	2.67	2.70	2.12	2.11	2.09	2.70	2.70	1.83	2.69
HCM2K98thQ:	0.0	0.0	0.0	0.9	0.0	24.1	24.4	25.5	0.0	0.0	48.1	0.3

Diamond Oaks TIAR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I

Level Of Service Detailed Computation Report

2000 HCM Operations Method (Future Volume Alternative)  
 Future Volume Alternative

Intersection #2 Caldwell Avenue/Ben Maddox Way

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

HCM Ops Adjusted Lane Utilization Module:

Lanes: 0 0 0 0 1 0 0 0 1 0 1 0 0 0 0 0 1 0 1

#LanesGrps: 0 0 0 0 1 0 0 1 1 0 1 1 1 0 0 1 1

Lane Width: 12 12 12 12 12 12 12 12 12 12 12 12 12 12 12 12 12 12

CrosswalkWid: 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8

3 Hev Veh: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Grade: 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%

Parking/Hr: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Bus Stop/Hr: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Area Type: < < < < < < < < < < < < < < < < < <

Chft Ped/Hr: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

ExcludesVeh: Include Include Include Include Include Include

3 RT Pctct: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

HCM Ops f(lt) Adj Case Module:

f(lt) Case: XXXX XXXX XXXX 1 XXXX XXXX 1 XXXX XXXX XXXX XXXX XXXX

HCM Ops Saturation Adj Module:

Ln Wid Adj: XXXX XXXX XXXX 1.00 XXXX 1.00 1.00 XXXX XXXX 1.00 1.00

Hev Veh Adj: XXXX XXXX XXXX 1.00 XXXX 1.00 1.00 XXXX XXXX 1.00 1.00

Grade Adj: XXXX XXXX XXXX 1.00 XXXX 1.00 1.00 XXXX XXXX 1.00 1.00

Parking Adj: XXXX XXXX XXXX XXXX XXXX 1.00 XXXX 1.00 XXXX XXXX 1.00

Bus Stop Adj: XXXX XXXX XXXX XXXX XXXX 1.00 XXXX 1.00 XXXX XXXX 1.00

Area Adj: XXXX XXXX XXXX XXXX XXXX 0.85 XXXX XXXX XXXX XXXX 0.95

RT Adj: XXXX XXXX XXXX XXXX XXXX 0.95 XXXX XXXX XXXX XXXX XXXX

LT Adj: XXXX XXXX XXXX XXXX XXXX 0.95 XXXX XXXX XXXX XXXX XXXX

Peak Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00 1.00 1.00

HCM Sat Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00 1.00

Off Sat Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Buf Sat Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00 1.00

Fnl Sat Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00 1.00

Delay Adjustment Factor Module:

Coordinated: < < < < < < < < < < < < < < < < < <

Signal Type: < < < < < < < < < < < < < < < < < <

DelAdjFctr: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Diamond Oaks TIAR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I

Level Of Service Detailed Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 Caldwell Avenue/Ben Maddox Way

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected Protected Protected

Rights: Include Include Include Include Include Include

Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0

Y+K: 0 0 0 0 1 0 0 0 1 0 1 0 0 0 0 1 0 1

Lanes: 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Base Vol: 0 0 0 0 12 0 253 258 691 0 0 781 8

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Base: 0 0 0 0 12 0 253 258 691 0 0 781 8

Added Vol: 0 0 0 0 0 0 17 9 8 0 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Ent: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Usr Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volumes: 0 0 0 0 13 0 283 290 760 0 0 866 9

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 0 0 0 0 13 0 293 290 760 0 0 866 9

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Buf Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Final Volume: 0 0 0 0 13 0 293 290 760 0 0 866 9

Saturation Flow Module:

Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00

Lanes: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00

Final Sat.: 0 0 0 1905 0 1615 1905 1900 0 1900 1615

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.18 0.16 0.40 0.00 0.00 0.46 0.01

Crit Moves: \*\*\*\*

Green/Cycle: 0.00 0.00 0.00 0.20 0.00 0.20 0.18 0.68 0.00 0.00 0.50 0.50

Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.21 0.91 0.59 0.00 0.00 0.91 0.01

Delay/Veh: 0.0 0.0 0.0 32.3 0.0 66.9 68.4 9.3 0.0 0.0 34.9 12.4

User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

AdjDel/Veh: 0.0 0.0 0.0 32.3 0.0 66.9 68.4 9.3 0.0 0.0 34.9 12.4

Diamond Oaks TIAR  
 55-2454-01/CN 1639  
 Existing PM Peak Hour + Phase I

Level Of Service Detailed Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #2 Caldwell Avenue/Ben Maddox Way

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected Protected Protected

Rights: Include Include Include Include Include Include

Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0

Y+K: 0 0 0 0 1 0 0 0 1 0 1 0 0 0 0 1 0 1

Lanes: 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Base Vol: 0 0 0 0 12 0 253 258 691 0 0 781 8

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Base: 0 0 0 0 12 0 253 258 691 0 0 781 8

Added Vol: 0 0 0 0 0 0 17 9 8 0 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Ent: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Usr Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volumes: 0 0 0 0 13 0 283 290 760 0 0 866 9

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 0 0 0 0 13 0 293 290 760 0 0 866 9

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Buf Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Final Volume: 0 0 0 0 13 0 293 290 760 0 0 866 9

Saturation Flow Module:

Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 0.95 1.00 1.00 1.00

Lanes: 0.00 0.00 0.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00

Final Sat.: 0 0 0 1905 0 1615 1905 1900 0 1900 1615

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.01 0.00 0.18 0.16 0.40 0.00 0.00 0.46 0.01

Crit Moves: \*\*\*\*

Green/Cycle: 0.00 0.00 0.00 0.20 0.00 0.20 0.18 0.68 0.00 0.00 0.50 0.50

Volume/Cap: 0.00 0.00 0.00 0.04 0.00 0.21 0.91 0.59 0.00 0.00 0.91 0.01

Delay/Veh: 0.0 0.0 0.0 32.3 0.0 66.9 68.4 9.3 0.0 0.0 34.9 12.4

User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

AdjDel/Veh: 0.0 0.0 0.0 32.3 0.0 66.9 68.4 9.3 0.0 0.0 34.9 12.4

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report (HCM 2000 Queue Method)

2000 HCM Operations Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Run Speed: 0.0 0.0 0.0 30 MPH 30 MPH 30 MPH 30 MPH 30 MPH  
NumOfStops: 0.0 0.0 0.0 2.6 0.0 71.7 71.1 101.0 0.0 198.1 1.1  
Name: Year 1995 composite fleet  
Fuel Consumption: 81.366 pounds  
Carbon Dioxide: 13.181 gallons  
Carbon Monoxide: 253.863 pounds  
Hydrocarbons: 20.359 pounds  
Nitrogen Oxides: 3.823 pounds  
Name: year 2000 composite fleet  
Fuel Consumption: 81.366 pounds  
Carbon Dioxide: 13.181 gallons  
Carbon Monoxide: 253.863 pounds  
Hydrocarbons: 20.359 pounds  
Nitrogen Oxides: 3.823 pounds

\*\*\*\*\*  
DISCLAIMER  
The fuel consumption and emissions measures should be used with caution and only for comparisons of different signal timings, geometric design alternatives or for general planning applications, as these calculations are applied to the analysis of a single intersection within the CCG and TRAFFIX. Network models are more appropriate since they can account for the influence of the adjacent control measures and other system elements.

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report (HCM 2000 Queue Method)

2000 HCM Operations Method  
Future Volume Alternative

\*\*\*\*\*  
Intersection #2 Caldwell Avenue/Ben Maddox Way  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R  
Green/Cycle: 0.00 0.00 0.00 0.20 0.00 0.20 0.18 0.68 0.00 0.00 0.50 0.50  
ActValType: 3 3 3 3 3 3 3 3 3 3 3 3  
ProgFactor: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Q1: 0.0 0.0 0.0 0.3 0.0 8.0 7.9 11.3 0.0 0.0 22.0 0.1  
UpstreamAdj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
EarlyAdmAdj: 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00  
Q2: 0.0 0.0 0.0 0.0 0.0 4.5 4.4 1.4 0.0 0.0 6.3 0.0  
HCM2Queue: 0.0 0.0 0.0 0.3 0.0 12.4 12.4 12.7 0.0 0.0 28.3 0.1  
70thFactor: 1.20 1.20 1.20 1.20 1.20 1.17 1.17 1.17 1.20 1.20 1.15 1.20  
HCM2F0thQ: 0.0 0.0 0.0 0.4 0.0 14.6 14.5 14.9 0.0 0.0 32.5 0.2  
85thFactor: 1.60 1.60 1.60 1.60 1.60 1.50 1.50 1.50 1.60 1.60 1.42 1.60  
HCM285thQ: 0.0 0.0 0.0 0.5 0.0 18.6 18.5 19.0 0.0 0.0 40.0 0.2  
90thFactor: 1.80 1.80 1.80 1.78 1.80 1.61 1.62 1.61 1.80 1.80 1.50 1.80  
HCM290thQ: 0.0 0.0 0.0 0.6 0.0 20.1 20.0 20.4 0.0 0.0 42.3 0.2  
95thFactor: 2.10 2.10 2.10 2.09 2.10 1.80 1.80 1.80 2.10 2.10 1.62 2.10  
HCM295thQ: 0.0 0.0 0.0 0.7 0.0 22.4 22.3 22.8 0.0 0.0 45.9 0.3  
98thFactor: 2.70 2.70 2.70 2.67 2.70 2.08 2.09 2.08 2.70 2.70 1.81 2.69  
HCM298thQ: 0.0 0.0 0.0 0.9 0.0 25.9 25.8 26.3 0.0 0.0 51.3 0.4

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report  
2000 HCM Unsignalized Method  
Base Volume Alternative

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

HevVeh: 0% 0% 0% 0%  
Grade: 0% 0% 0% 0%  
Pedestrian Walk Speed: 4.00 feet/sec 0 0  
Lanes: 12 feet 12 feet 12 feet 12 feet  
Time Period: 0.25 hour  
Upstream Signals:  
Link Index: #6  
Dist(miles): 0.00  
Speed (mph): 0.00  
SignalIndex: #2  
Cycle Time: 0 secs  
InitVolume: 0 0 0 0  
Saturation: 0 0 0 0  
ArrivalType: 0 0 0 0  
G/C: 0.00 0.00  
P: 0.00 0.00  
Sg1: 0.00 0.00  
Sg2: 0.00 0.00  
alpha: 0.00  
beta: 0.00  
tau (secs): 0.00  
f: 0.00 0.00  
vcmax: 0 0  
vcg: 0 0  
lp: 0.0 0.0  
P: 0.000

\*\*\* Computation 1: Time for Queue to Clear at Each Upstream Intersection  
P: 0.00 0.00  
Sg1: 0.00 0.00  
Sg2: 0.00 0.00  
\*\*\* Computation 2: Time Intersection Blocked Because of Upstream Platoons  
alpha: 0.00  
beta: 0.00  
tau (secs): 0.00  
f: 0.00 0.00  
vcmax: 0 0  
vcg: 0 0  
lp: 0.0 0.0  
P: 0.000  
Pform/Prob: 0.000/0.000/Unconstrained  
\*\*\* Computation 4: Conflicting Flows During Each Unblocked Period  
InitCtrlVol: 2173 1032 2173 2173 1141 0 XXXXX XXXXX 0 XXXXX XXXXX  
AdjCtrlVol: 2173 2173 1032 2173 2173 1141 0 XXXXX XXXXX 0 XXXXX XXXXX  
UpstreamAdj: 1.00 1.000 1.000 1.000 1.000 1.000 1.00 X.XXX X.XXX 1.00 X.XXX X.XXX  
ConflictVol: 2173 1032 2173 2173 1141 0 XXXXX XXXXX 0 XXXXX XXXXX  
\*\*\* Computation 5: Capacity for Subject Movement During Unblocked Period  
InitPotCap: 52 47 285 52 47 246 1636 XXXXX XXXXX 1636 XXXXX XXXXX  
UpstreamAdj: 1.00 1.000 1.000 1.000 1.000 1.000 X.XXX X.XXX 1.00 X.XXX X.XXX  
Potent Cap.: 52 47 285 52 47 246 1636 XXXXX XXXXX 1636 XXXXX XXXXX

Diamond Oaks TIAR  
55-2454-01/CN 1639  
Existing PM Peak Hour + Phase I

Level of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

\*\*\*\*\*  
Intersection #3 Caldwell Avenue/Edison Street  
\*\*\*\*\*  
Average Delay (sec/veh): 0.0 Worst Case Level of Service: A| 0.0|  
\*\*\*\*\*  
Approach: North Bound South Bound East Bound West Bound  
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled  
Rights: Include Include Include Include  
Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0  
Volume Module:  
Base Vol: 0 0 0 0 0 0 0 0 949 0 0 1050 0  
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
Initial Bse: 0 0 0 0 0 0 0 0 949 0 0 1050 0  
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
PHE Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92  
PHE Volume: 0 0 0 0 0 0 0 0 1032 0 0 1141 0  
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0  
FinalVolume: 0 0 0 0 0 0 0 0 1032 0 0 1141 0

Critical Gap Module:  
Critical Gap: XXXX XXXX 5.2 XXXX XXXX XXXXX XXXX XXXX XXXX XXXX XXXX  
FollowUpTime: XXXX XXXX 3.5 XXXX XXXX XXXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Capacity Module:  
Conflict Vol: XXXX XXXX 1032 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Potent Cap.: XXXX XXXX 285 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Volume/Cap.: XXXX XXXX 285 XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Level of Service Module:  
2Way5Thq: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Control Del: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
LOS by Move: \* \* \* \* \*  
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT  
Shared Cap.: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
SharedQueue: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shrd ConDel: XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX  
Shared LOS: \* \* \* \* \*  
ApproachDel: XXXXX \* XXXXX \* XXXXX \* XXXXX \*  
ApproachLOS: \* \* \* \* \*

Note: Queue reported is the number of cars per lane.  
\*\*\*\*\*

Diamond Oaks TIA#
55-2454-01/CN 1639
Existing PM Peak Hour + Phase I

Level of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)
Intersection #3 Caldwell Avenue/Edison Street

Average Delay (sec/veh): 0.1 Worst Case Level of Service: C1 (18.3)
Approach: North Bound South Bound East Bound West Bound
Movement: L T R L T R L T R L T R L T R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Right: Include Include Include Include
Lanes: 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 1 0 0
Volume Module:
Base Vol: 0 0 0 0 0 0 0 0 949 0 0 1050 0
Greench Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Critical Gap Module:
Critical Gap: XXXXX XXXX 6.2 XXXXX XXXX XXXXX XXXX XXXX XXXX XXXX
Followup Time: XXXX XXXX 3.3 XXXXX XXXX XXXXX XXXX XXXX XXXX XXXX XXXX XXXX
Capacity Module:
Conflict Vol: XXXX XXXX 1043 XXXX XXXX XXXXX XXXX XXXX XXXX XXXX XXXX XXXX
Potential Cap: XXX XXXX 281 XXXX XXXX XXXXX XXXX XXXX XXXX XXXX XXXX XXXX XXXX

Diamond Oaks TIA#
55-2454-01/CN 1639
Existing PM Peak Hour + Phase I

Level of Service Detailed Computation Report
2000 HCM Unsignalized Method
Future Volume Alternative
Intersection #3 Caldwell Avenue/Edison Street

Approach: North Bound South Bound East Bound West Bound
Movement: L T R L T R L T R L T R
Heavy Veh: 0% 0% 0% 0%
Grade: 0% 0% 0% 0%
Peds/Hour: 0 0 0 0
Pedestrian Walk Speed: 4.00 feet/sec 12 feet 12 feet 12 feet

Upstream Signals:
Link Index: #6
Dist (miles): 0.00
Speed (mph): #2
Signal Index:
Cycle Time: 0 0 0 0
Init Volume: 0.00 0.00
Saturation: 0.00 0.00
Arrival Type:
G/C: 0.00 0.00
\*\*\* Computation 1: Time for Queue to Clear at Each Upstream Intersection

991: 0.00 0.00
992: 0.00 0.00
993: 0.00 0.00
\*\*\* Computation 2: Time Intersection Blocked Because of Upstream Platoons
alpha: 0.000 0.000
beta: 0.000 0.000
ta (secs): 0.000 0.000
F: 0.000 0.000
vcmax: 0 0
vcq: 0 0
vcmin: 0 0
lp: 0.0 0.0
\*\*\* Computation 3: Platoon Event Periods
pdown/upside: 0.000/0.000/unconstrained